Work Plan

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## Remedial Investigation/ Feasibility Study

National Smelting of New Jersey Site Pedricktown, New Jersey

NL Industries Hightstown, New Jersey

May 1987



#### REMEDIAL INVESTIGATION/FEASIBILITY STUDY (RI/FS) SCHEDULE

SPRING 1988: FIELD ACTIVITIES COMMENCE

SUMMER 1988:

ROUND ONE SAMPLING PROGRAM

11 SURFACE WATER SAMPLES 36 GROUNDWATER SAMPLES

159 SOIL AND SEDIMENT SAMPLES

58 WASTE MATERIAL SAMPLES

FALL 1988:

ROUND TWO SAMPLING PROGRAM

11 SURFACE WATER SAMPLES

36 GROUNDWATER SAMPLES

SPRING 1989:

FINAL RI REPORT

SPRING 1990: FINAL RI/FS REPORT

#### WORK PLAN

# REMEDIAL INVESTIGATION/FEASIBILITY STUDY NATIONAL SMELTING OF NEW JERSEY SITE PEDRICKTOWN, NEW JERSEY

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#### SECTION 1 - STUDY AREA

#### 1.01 Regional Description

The secondary lead smelting facility formerly owned by NL Industries, Inc. and owned by National Smelting of New Jersey, Inc. (NSNJ) is located in Oldman's Township, Salem County, New Jersey. Figure 1 presents a location map for the site.

The topography of the region that includes the site is flat to gently rolling land which slopes to the north. The site is approximately ten feet above sea level. The Delaware River lies approximately 1.5 miles to the northwest of the site.

The regional geology is characterized by surficial deposits belonging to the Cape May formation. These deposits consist of fine to medium sand, which is occasionally coarse grained. The deposits extend to a maximum depth of approximately thirty feet and typically yield limited volumes of water. Dense red and gray clays belonging to the Raritan formation separates the surficial formation from the first confined aquifer, a sand unit of the Raritan-Magothy formation. A second clay layer separates the first confined aquifer from the second confined layer. The depth to bedrock may be about three hundred feet. The above information is detailed in Geraghty & Miller, Inc.'s report entitled "Hydrogeologic Study and Design of Groundwater Abatement System at NL Industries, Inc., Pedricktown, New Jersey Plant Site" (May, 1983).

#### 1.02 Oldman's Township

Oldman's township is located in the northern portion of Salem County. It is bounded on the north and west by the Delaware River, by Oldman's Creek and Glouster County on the north and east, and Upper Penns Neck Township on the south. The site is located along Penns Grove-Pedricktown Road in Pedricktown as shown in Figure 2.

The site is part of an area zoned for development as an industrial park and includes operations of the following major corporate entities: Airco, B.F. Goodrich, Browning-Ferris Industries and Exxon, Tomah Division. It has been reported that some of the industries may have addressed environmental concerns on their property.

#### 1.03 NSNJ Property

The site includes the following parcels of property as presented in Figure 2:

a. Thirty acres of land southeast of the Pennsylvania-Reading Seashore Lines railroad tracks that include Block 37, Lot 2 (i.e., the "plant area") that includes the former secondary lead smelter.

b. Sixteen acres of land northwest of the Pennsylvania-Reading Seashore Lines railroad-tracks that include Block 39, Lots 17 and 21 (i.e., the "landfill area") that incorporates a closed landfill.

#### SECTION 2 - SITE HISTORY

#### 2.01 Site Development

The Pedricktown secondary lead smelter was constructed in 1971-2. The smelter originally made use of a blast furnace and a reverberatory furnace for smelting. A sweater furnace was also on site for melting of elemental lead scrap. The Pedricktown facility was upgraded to incorporate systems that would do the following:

- a. Take a tractor-trailer loaded with scrap batteries and dump the scrap batteries into an acid brick lined bin by inclining the tractor trailer to a sixty degree angle on a hydraulic ramp.
- b. Crush the batteries.
- c. Separate the plastic/rubber case materials, metallic lead, and lead compounds for recycling.
- d. Smelt lead bearing materials (i.e., a rotary kiln) with minimal emissions of sulfur oxides.

A detailed drawing of the plant area showing major pieces of equipment and production areas is presented in Figure 3.

The Pedricktown refinery was built in 1972. The facility consisted of twelve refining kettles and a one hundred mold Wirtz casting machine. The refinery produced soft lead and antimonial lead from the furnace product.

#### 2.02 Rotary Kiln Installation and Process

Secondary lead smelting at the Pedricktown facility originally was accomplished using a blast furnace and a reverberatory furnace. These were taken out of service when a rotary kiln was installed in 1978. The rotary kiln employed at NL Industries' Pedricktown facility is a 177 foot long, 10 foot diameter converted cement kiln. A thirty-five million BTU oil burner fires the kiln at the discharge end. The rotary kiln is located east of the slag yard and slag crusher building on the west side of the plant area.

Raw materials fed into the kiln consisted of soda ash, cast iron chips, coke and lead bearing feed. All raw materials were fed to the rotary kiln by a 150 foot long completely enclosed belt conveyor. The material was transferred from the top of the conveyor belt and discharged into the rotary kiln by either an angled feed chute or a feed screw. The device used depended on the type of material entering

the kiln. The gas stream from the rotary kiln was sent through an eighteen cell baghouse for particulate removal.

The slag and metal collected at the discharge end of the rotary kiln. Slag continually overflowed a corebell weir at the discharge end of the kiln and was deposited in slag pots via chutes. Lead was taken periodically from the kiln by stopping the kiln, breaking the tap hole open and rotating the kiln until the tap hole allowed lead to flow down a refractory lined chute into molds. The slag pots and lead button molds were placed on the cable-drawn trains. Pellet/middlings, plant scrap, and 3:1 grid metal/pellets were the three basic charges to the kiln. The kiln output varied depending on the charge used.

A more detailed account of rotary kiln operation is provided in "Rotary Kiln Smelting Of Secondary Lead", by R.C. Egan, M.V. Rao and K.D. Libsch (Reprinted from: Lead-Zinc-Tin '80, edited by John M. Cigan, Thomas S. Mackey and Thomas J. O'Keefe, Proceedings of a World Symposium on Metallurgy and Environmental Control sponsored by the TMS-AIME Lead, Zinc and Tin Committee at the 109th AIME Annual Meeting, February 24-28, 1980, Las Vegas, Nevada).

#### 2.03 Landfill Development and Closure

NL Industries, Inc. (NL) established a permitted hazardous waste landfill on its Pedricktown facility's property. Figure 2 shows the location of the landfill, which consists of two phases, Landfill Phase A and Landfill Phase B. Landfill Phase A contains process wastes (blast furnace and kiln slag) from the facility, while Landfill Phase B also contains hard rubber case material and lead contaminated soils that were excavated from the facility's grounds. Details on landfill design and closure are presented as Exhibit A.

#### 2.04 1982-83 Cleanup and Property Transfer to NSNJ

NL Industries, Inc. (NL) terminated lead production using the rotary kiln on May 27, 1982. On October 6, 1982, NL signed an Administrative Consent Order (ACO) with the New Jersey Department of Environmental Protection (NJDEP) whereby NL agreed to undertake a variety of activities in order to address environmental conditions at the site. The activities under the ACO involved the following:

- removal of specific marsh soils,
- oprevention of runoff of all surface water from paved areas,

- ° cleaning of the paved areas shown in Figure 5,
- retention of a consulting firm (Roy F. Weston, Inc.) to prepare closure and post-closure plans that would comply with 40 CFR 265,
- installation of two deep groundwater monitoring wells,
- ° sampling on a quarterly basis of groundwater monitoring wells, and
- eretention of consulting firms to design a groundwater abatement system (Geraghty and Miller, Inc.) and installation and testing of the groundwater abatement system (Moretrench American Corp. and Ground Water Technology, Inc.).

NSNJ exhibited interest in purchasing and operating the Pedricktown facility. National Smelting and Refining Co., Inc. (NSR) was a part owner in NSNJ. NL, NSNJ, NSR and NJDEP entered into an Amended Administrative Consent Order (AACO) on February 10, 1983 to delineate and distribute the environmental obligations specified in the ACO. The AACO amended the ACO, transferring to NSNJ environmental obligations which were originally the responsibility of NL under the ACO. NSNJ agreed to comply with ongoing environmental requirements, while NL still retained some of its original obligations. An agreement for sale of the Pedricktown lead smelting facility between NL and NSNJ was dated February 24, 1983. Prior to February 8, 1983 NL had completed all cleanup measures directed at the plant area. NL prepared photographic documentation of the work that was completed.

The NJDEP acknowledged NL's completion of specified items under the ACO in the preamble to the AACO.

#### 2.05 NSNJ Operations

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NSNJ took possession of the Pedricktown property on February 24, 1983. NSNJ then operated the facility until January 20, 1984. NSNJ commenced rotary kiln smelting on May 27, 1983. NSNJ filed for bankruptcy under Chapters 11 and 7 on March 5th and 27th 1984 respectively.

During the operation of the Pedricktown facility by NSNJ, NSNJ allowed slag waste from their processing of lead, along with other bulk, drummed and/or containerized waste materials and raw materials (including ore concentrates, fluxes and reagents) to accumulate in non-enclosed areas that were exposed to the elements. Following bankruptcy filing, the National Bank of Georgia, trustee for the site bond holders, maintained environmental personnel at the site for landfill maintenance purposes until June 15, 1984. NL voluntarily entered the site on June 18, 1984 to pump landfill leachate which had accumulated in the leachate sumps, and to maintain landfill cover materials.

#### SECTION 3 - EXISTING CONDITIONS

#### 3.01 Existing Facilities and Utilities

The following facilities and utilities are now present at the Pedricktown smelting facility:

- a. Battery yard.
  - i. battery hopper and breaker (shredder)
  - ii. acid handling facilities
  - iii. spent battery storage
- b. Processing equipment for lead compounds to render them suitable for furnace feed.
  - i. Trommel
  - ii. Bucketwheel
  - iii. Split Screen
  - iv. Spiral Classifier
  - v. Libra Screen
  - vi. Vacuum Filter
  - vii. Dryer (pelletizer)
- c. Rotary Kiln furnace and related equipment (buffer storage buildings, feed conveyor, coke and soda ash silos, iron hopper, slag and lead trains, slag storage bins, and slag crusher).
- d. "Sweater" furnace to melt elemental lead scrap.
- e. Refining Kettles.
- f. Old Blast Furnace.
- g. Casting lines for crude and purified ingots.
- h. Emission control equipment consisting of the following:
  - i. "Fuchs "chamber
  - ii. "Balloon duct" for rotary kiln
  - iii. "Metallurgical baghouse" for rotary kiln
  - iv. "Sanitary baghouse and two cyclones" for refining building
  - v. Two "sweater furnace baghouses" and a cyclone
  - vi. "Slag crusher baghouse"
  - vii. "Toy baghouses"
  - viii. "Ducon wet scrubber"
  - ix. Related ductwork and hoods

Currently there is no electrical power supplied to the production areas. Electrical power is supplied to maintain the landfill.

#### 3.02 Existing Material Storage

The following material storage facilities currently exist at the Pedricktown smelting facility:

- a. A large receiving yard (acid brick lined) which contains various lead bearing materials and waste materials.
- b. A warehouse which contains a variety of drummed materials. Two silos containing bulk soda ash and petroleum coke. The drummed or otherwise containerized raw materials which are located in the warehouse consist of: caustic soda, sodium nitrate, red phosphorus, sulfur, lime, saw dust, charcoal, caustic potash, metallic cadmium, selenium, copper, arsenic, bismuth, calcium, calcium/aluminum, tin/aluminum, sodium, antimony, tin, zinc and aluminum.
- c. An inner yard which contains various lead bearing raw and waste materials.

  The inner yard was intended for the temporary storage of crude metallic lead

  "buttons" from the kiln and sweater furnaces.
- d. Bins containing various lead bearing and waste materials. The bins were intended for the storage of "dross", iron, coke and slag.
- e. An acid pit and two acid tanks.
- f. A thickener pit and tank.
- g. Wastewater tanks.
- h. Effluent tank.
- i. Wash water tank.

#### 3.03 Current Landfill Maintenance Activities and Facilities

Presently, the activities associated with the landfill include cover maintenance (e.g. mowing, visual inspections) and leachate management (e.g. leachate monitoring and pumping and coordination of leachate hauling).

Facilities presently found at the landfill consist of the closed hazardous waste landfill, the landfill office and maintenance facilities and the diked leachate storage tank.

#### SECTION 4 - PRELIMINARY REMEDIAL TECHNOLOGIES

Prior to starting any investigations, conditions at the site will be addressed relative to risks associated with the no-action alternative. Should the projected risk be such that some action is justified, the appropriate actions will be evaluated.

This exercise will also identify those categories of remedial technologies that are potentially applicable to remedy adverse environmental conditions at the site. In this way, the Remedial Investigation can be tailored to ensure that the field investigation will generate sufficient data to properly evaluate the technologies and alternatives in the Feasibility Study.

#### 4.01 Interim Remedial Measures

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The possible use of an interim remedial measure will be based on the evaluation of risks associated with the no action alternative. Interim remedial measures considered for the Pedricktown facility consist of a variety of alternatives designed to restrict transportation of material from the site in the near term. For example, existing storage structures could be utilized for materials that should be kept out of the elements. In addition, covers might also be used to protect bulk materials from rain and wind that could transfer the material from the site into surrounding areas. The evaluation of interim remedial measures and recommendations associated with these measures will be presented in the Draft Remedial Investigation Report.

#### 4.02 Recycle/Reuse Assessment

The possibility of recycling and/or reusing materials found at the Pedricktown smelting facility will be considered as a preliminary remedial technology. Many of the materials accumulated on the site are raw or intermediate materials. Consequently, recycling and/or reusing the materials is expected to play a major role in site remediation. Consideration will be given to methods which would effectively recycle or reuse materials found at the Pedricktown site.

#### 4.03 On-Site Remedial Approaches

Several on-site remedial approaches exist for the Pedricktown smelting facility. These include the excavation of bulk materials and contaminated soils. The excavated material could be placed in containers for storage on-site, they could be solidified on-site, or they could be landfilled on-site. Demolition of selected structures contained on the site could also be done. Groundwater controls such as pumping and treatment or impermeable barrier walls could be considered in the case of contaminated groundwaters. At the present time, a ground water abatement system exists at the site, although no method of treatment has been designed and constructed. In situ fixation of soils could also be considered.

#### 4.04 Off-Site Remedial Approaches

As with on-site remedial approaches, off-site remedial approaches to be considered include off-site treatment and disposal of excavated bulk materials and soils. The excavated materials could be landfilled off-site. Fixation/solidification techniques may be used prior to off-site land disposal. Recycling and reuse of on-site materials could also be considered.

#### SECTION 5 - REMEDIAL INVESTIGATION/FEASIBILITY STUDY OBJECTIVES

The Remedial Investigation/Feasibility Study is designed to accomplish the following goals:

- A. Identify the environmental conditions at the NSNJ property and surrounding properties affected by the Site.
- B. Evaluate the impacts that any past, present or future release or migration of contaminant may have on public health or the environment.
- C. Develop, screen, and evaluate potential response actions in accordance with the National Oil and Hazardous Substances Pollution Contingency Plan (NCP), 40 CFR Section 300.68.

Table 11 presents the anticipated schedule for the Remedial Investigation/Feasibility Study.

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## TABLE 11 NSNJ PEDRICKTOWN REMEDIAL INVESTIGATION/FEASIBILITY STUDY SCHEDULE

	Task	<u>Duration</u> <sup>1</sup>	Predecessor
1.	Work Plan Preparation		
2.	Operations Plan Preparation	30 days	1
3.	Access Obtained, Safety Survey	60 days	Approval of 2
4.	Field Investigations-First	30 days	2, 3
5.	Laboratory Analyses	40 days	4
6.	Data Analysis/Interim Report		4, 5 John 1/16/29
7.	Field Investigation-Second	30 days	Approval of 6 Endel 16/18
8.	Laboratory Analyses	40 days	7 26 114 40
9.	Data Analysis and Draft RI Report	90 days	8 44, 45, 40
10.	Final RI Report	30 days	Receipt of Comments on 9
×11.	Remedial Alternatives Development	30 days	9
12.	Remedial Alternatives Screening	30 days	10
13.	Remedial Alternatives Evaluation	90 days	Approval of 10
14.	Draft RI/FS Report	60 days	13
15.	Final RI/FS Report	30 days	Receipt of Comments on 14
		500	and days on the same

(1) Working Days - excludes public holidays and weekends

## TABLE 11 NSNJ PEDRICKTOWN REMEDIAL INVESTIGATION/FEASIBILITY STUDY SCHEDULE

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6.	Data Analysis/Interim Report	30 days	4, 5
7.	Field Investigation-Second	30 days	Approval of 6
8.	Laboratory Analyses	40 days	7
9.	Data Analysis and Draft RI Report	90 days	8 .
10.	Final RI Report	30 days	Receipt of Comments on 9
11.	Remedial Alternatives Development	30 days	9
12.	Remedial Alternatives Screening	30 days	10
13.	Remedial Alternatives Evaluation	90 days	Approval of 10
14.	Draft RI/FS Report	60 days	13
15.	Final RI/FS Report	30 days	Receipt of Comments on 14
		530 No.	- Li days ~ 2,12 winey

(1) Working Days - excludes public holidays and weekends

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#### SECTION 6 - REMEDIAL INVESTIGATION WORK PLAN

#### 6.01 Site Investigations

Several studies have been conducted from 1980 through 1986 involving soils, surface water, and ground water at the Pedricktown site. The purpose of this section is to present existing data and outline what activities will be conducted as part of the Remedial Investigation. It should be noted that the NJDEP has conducted several studies for which data are not currently available. The scope of work presented below may be reduced upon review of the NJDEP data.

#### 6.01.1 Soils Investigation

#### Previous Studies:

A soil sampling program was completed by NL in early 1981 which was conducted in response to concerns raised by the NJDEP. The samples were obtained in late 1980. Sample locations from this study are shown in Figure 6. Total lead analysis was run for all samples, with the sample depths analyzed varying between sample locations. Samples were obtained from depths of 0"-2", 5"-7" and 11"-13". The results of these analyses are presented in Table 1.

Sediment samples were also obtained from marsh areas in late 1980 as part of the overall site soil sampling program. Sediment sample locations are presented in Figure 7. Total lead analysis was conducted on all samples, with sample depth analyzed varying between sample locations. Sample depths and corresponding analytical results are presented in Table 2.

These analyses were the basis for the excavation of contaminated plant soils and marsh sediments which took place prior to the sale of the facility. As mentioned in Section 2.4, areas and depths of excavation are presented in Figure 4. The excavated soils and sediments were placed in the Phase B landfill located on-site, which was subsequently closed in accordance with an approved closure plan.

#### Develop Sampling Grid:

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A sampling grid has been developed to locate surface soil sampling points. By utilizing a grid pattern, the areal distribution of contaminants can be readily identified. A regular grid pattern also allows the use of interpolation techniques to identify concentrations of contaminants between sampling points.

Studies have indicated that surface soil lead concentrations decrease exponentially with distance from the source such that concentrations of lead less than 1000 ppm would typically be present at distances greater than approximately 1200 feet from a source such as a secondary lead smelter (Roberts, TM, T.C. Hutchinson, et al. 1974. "Lead Contamination Around Secondary Smelters: Estimation of Dispersal and Accumulation by Humans." Science 186:1120-22.). Lead concentration in surface soils would be expected to be highest and most variable near the source (i.e., the site). As the distance from the source increases, the lead concentration in the surface soil would be expected to decrease and less variation in the lead concentrations would also be observed. Therefore, the surface soil sampling grid will consist of a finer grid pattern on the site and a progressively coarser pattern as the distance from the site increases.

A regularly spaced triangular grid pattern will be utilized in determining on and off-site soil sampling points. A triangular grid pattern is more efficient relative to sample area coverage than is a rectangular grid pattern (Parkhurst, D.F., "Optimal Sampling Geometry for Hazardous Waste Sites", Environmental Science and Technology, 1984, 18, 521-523). Two hundred foot triangles will be utilized within the property lines of the facility. Outside of the facility boundaries, two sets of four hundred foot triangles will be used, followed by a single set of eight hundred foot triangles. This will provide for characterization of surface soil concentrations at distances from the facility boundaries of 1600 to 2000 feet, which represents distances of approximately 2000 to 2500 feet from the source. The surface soil sampling locations are presented in Figure 2. Supplemental sampling of the marsh area is addressed in Section 6.01.3.

#### Brief Description of Sampling Protocol:

Each grid point sample will be composed of four discrete samples collected from around the grid point and composited. A three meter diameter circle will be measured around the grid point and samples will be taken from the northernmost point on the circle, the southernmost point, the easternmost point and the westernmost point and then composited. In the event that a three meter circle cannot be utilized around the grid point, four discrete samples will be collected along a line extending approximately twenty feet from the grid point and then composited. Composite samples will be collected to represent strata of 0" to 3", 3" to 6", 6" to 12", and 12" to 18" below grade. Soil samples from the secure landfill cover shall be to a depth of 18 inches or to the clay layer, whichever is least. All surface soil samples will be collected by hand driven 3/4" Lexan<sup>®</sup> tubes. Exact sample locations will be determined in the field. Every effort will be made to avoid collecting soil samples that are less than twenty feet from painted surfaces and/or under or immediately adjacent to trees, shrubs and/or structures. Collection sites will also be located as far as possible from vehicle activity such as streets, driveways, parking areas and automobile repair areas. Detailed sampling protocols will be included in the Project Sampling Plan which will be part of the Site Operations Plan.

#### Analytical Parameters:

Soil samples from 0" to 3" and 3" to 6" below grade will be analyzed for total lead. Approximately 10% of the soil samples will be analyzed for supplemental metals consisting of antimony, arsenic, cadmium, copper, chromium, lead, selenium, tin, and zinc. Approximately 75% of the samples to be tested for the supplemental metals will be selected from on-site locations. If the 3" to 6" below grade sample demonstrates contamination then the two deeper samples at that location will be digested and analyzed for only those parameters above background. Examination of past data suggests considerable variance for lead concentration in soil. For the purpose of this investigation deeper samples will be analyzed if the 3" to 6" strata has a total lead concentration (dry weight basis) of greater than 200 ppm. The analytical program for soil samples is presented in Table 3.

#### 6.01.2 Stored Materials Investigation

#### Previous Studies:

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Previous studies regarding bulk and containerized solids are limited to analyses of rotary furnace slag. Analyses have previously been conducted on the landfill leachate. This leachate data is presented in Exhibit D.

Bulk and Containerized Solids Preliminary Inventory:

A large quantity of bulk and containerized materials exist at NSNJ's Pedricktown facility. The bulk and containerized solids consist of: slag, equipment residue and containerized solids (i.e., baghouse dust, miscellaneous process waste and raw materials). These materials are present in the plant area and warehouse. An inventory of these materials will be conducted to quantify the amounts of these materials present at the facility and to identify their locations on the site. Mr. Stephen W. Holt, the individual who was responsible for the facility's environmental activities from March 1979 to February 1983, will assist in identifying materials during the inventory based on his experience at the site.

#### Characterization Objectives:

Each group of bulk and containerized materials identified at the facility will be sampled, with the exception of labeled containerized raw material and specifically identifiable bulk materials (i.e. new refractory brick, used bags, pellets, etc.). Analyses will be run on the samples of unidentified materials and identified materials to classify them as hazardous or non-hazardous. Knowledge of the composition and characteristics of identified materials may be utilized in lieu of analysis. The objectives of analyzing identified materials which may be hazardous is to determine appropriate management approaches. For example, lead bearing ceramic industry waste would be analyzed for total lead to determine feasibility of recycle operations. Only total lead analysis will be conducted on the equipment residue samples, since they are essentially raw and intermediate materials and will likely be recycled. The characterization of the materials as hazardous or non-hazardous will determine the method of management for each type of material.

#### Brief Description of Sampling Protocol:

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Samples will be taken from bulk and containerized raw materials identified, with the exception of labeled containerized raw materials. Based on the available estimates the following samples will be collected:

three composite slag samples one each from the iron and coke bins, battery bins, and slag bins,

- ° ten equipment residue samples, and
- \* twenty-five containerized solids samples.

The slag samples will be collected manually using geologic tools or other equipment as necessary. Samples from the other areas will be collected with shovels or other appropriate tools. Drum and container sampling will be accomplished by using a sampling trier or other appropriate equipment. The Sampling Plan will present details of how this will be accomplished.

#### Analytical Parameters:

Total lead analysis will be conducted on all bulk and containerized solids samples. The EP toxicity test for all metals listed in 40 CFR 261.24 will be run on all slag samples. In addition, metal analyses for the following metals: antimony, arsenic, cadmium, chromium, copper, selenium, tin, and zinc will be conducted on unknown bulk and containerized solids samples. The analytical program for bulk and containerized solid materials is presented in Table 3.

#### Contained Liquid:

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The liquid volume of stormwater and wastewater contained in the following areas will be estimated: a pond on asphalt pavement at the east side of the plant area, a pond on concrete pavement in the center of the plant area, an acid pit, two acid tanks, a thickener pit, a thickener tank, wastewater tanks, an effluent tank, leachate sumps for landfill Phases A and B, primary and secondary and a washwater tank. These facilities are identified on the plant area map presented as Figure 3. Miscellaneous accumulations (less than 5000 gallons) exclusive of drums or tanks will be pumped to one of the above areas prior to any inventories and sampling.

#### Brief Description of Sampling Protocol:

One sample will be taken from each of the storage areas/facilities noted previously as holding storm or wastewater. If the liquid depth at the sample location is greater than three feet, a depth compositing technique will be used to obtain the samples. Otherwise, grab sampling techniques will be utilized to obtain the samples.

Rainwater accumulations in uncovered drums will be pumped to a storage container and sampled as a composite. The uncovered drums will be covered after pumping off the accumulated rainwater. All unidentified covered drums containing liquids will be sampled.

#### Analytical Parameters:

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Each sample taken will be analyzed for pH, lead, and total organic carbon (TOC). Leachate, if present, from primary and secondary sumps of landfill Phases A and B will also be analyzed for Total Organic Carbon, Total Organic Halogens, Gross Alpha and Beta Radiation, cyanides, and priority pollutant metals. If Total Organic Carbon or Total Organic Halogen results indicate a potential problem then priority pollutant organic analyses will be conducted. Analytical results from leachate samples will be compared to those for hydraulically upgradient and downgradient ground water monitoring wells in the vicinity of the landfill to determine whether migration of contaminants from the landfill is occurring.

#### 6.01.3 Surface Water and Marsh Investigation

#### Adjacent Surface Waters

As indicated on the topographic map and site map presented in Figures 1 and 2, respectively, a stream courses along the site's western boundary. The stream, referred to as the West Stream, receives most of the stormwater runoff from the site and eventually discharges into the Delaware River.

A marshy area which intermittently holds surface water is present on the site as shown in Figure 2. One portion of the marshy area is south of the railroad tracks (i.e., the "south marsh") and one portion is north of the railroad tracks (i.e., the "north marsh"). The north marsh and south marsh are hydraulically connected by a culvert which passes beneath the railroad tracks. Stormwater from several sections of the plant area runs off into the south marsh and through the culvert into the north marsh. An intermittent stream runs from the north marsh to the West Stream, so when sufficient surface water is in the north marsh, it discharges into the West Stream and eventually into the Delaware River.

A second stream, located on Figure 2, runs approximately 1000 feet east of and parallel to the site's eastern property boundary. This stream,

referred to as the East Stream, receives stormwater runoff from surrounding properties. During periods of high flow, the East Stream reportedly backs up into an intermittent channel which discharges into the south marsh.

#### Previous Studies:

Water samples from the marsh area were collected at various times during the period from 1981 through 1983 and analyzed for a variety of parameters. The analytical results for these samples are presented in Table 5. The data indicate that several parameters were detected in levels exceeding Water Quality Criteria. No previous studies of the East or West Streams are known to have occurred.

Sediment samples collected from the marsh area during 1980 were analyzed for lead to determine limits of excavation. A brief description of the results is presented in Section 6.01.1 and Table 2.

#### Sampling Rationale:

In order to characterize surface water quality, particularly relative to site impacts on surface water quality, samples should be obtained upgradient of the site, on or adjacent to the site, and downgradient of the site. The objective would be to sample during both high and low flow conditions to account for water quality variations with stream flow.

To evaluate the potential for sediment transport, sediment samples will be collected at eight locations in the marsh area and at each surface water sampling location.

#### Approximate Sample Locations and Frequency:

Surface water and sediment samples will be obtained at several locations. Approximate sample locations are presented on Figure 8.

Regarding the West Stream, samples will be obtained from each of the following locations:

- upstream of the facility's western property boundary
- immediately downstream of the facility's western property boundary
- approximately 800 feet downstream of the facility's western property boundary.

The samples from the East Stream are located such that samples will be obtained from each of the following areas:

- immediately upstream of the railroad track
- approximately 1000 feet downstream of the railroad track.

Surface water samples will be collected twice during the course of the Remedial Investigation to account for varying environmental conditions.

Approximate locations for the eight sediment samples from the marsh are presented on Figure 2.

#### Brief Description of Sampling Protocol:

Surface water samples will be obtained using grab sampling techniques in the approximate middle of the channel approximately one inch below the surface, in such a way so as to avoid suspension of sediments.

Sediment samples will be collected using a coring device to represent the strata from 0" to 1" below the top of sediment. Specific sampling procedures will be detailed in the Sampling Plan.

#### Analytical Parameters:

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All surface water samples will be analyzed for pH and total lead. In addition, the sample from the location immediately upstream of the intermittent stream's discharge into the west stream will be analyzed for the following total metals: antimony, arsenic, cadmium, chromium, copper, selenium, tin, and zinc.

Sediment samples will all be analyzed for total lead. Sediment collected immediately upstream of the intermittent stream's discharge into the West Stream will be analyzed for the same metals listed for that surface water sample. The analytical program for surface water samples is presented in Table 3.

#### 6.01.4 Hydrogeologic Investigation

Previous hydrogeologic studies have demonstrated the existence of three water bearing units beneath the site. The three units consist of: the water table aquifer, first confined aquifer, and second confined aquifer. Each strata

is described separately below with a review of previous studies and proposed additional activities. A summary of ground water data generated since 1981 for both the on-site wells and nearby private wells is presented in Tables 6 through 9. Exhibit E presents ground water level measurements obtained during previous studies.

#### Water Table Aquifer

#### Previous Studies:

The water table aquifer directly beneath the Pedricktown facility is of the Cape May Formation and is composed mainly of fine to medium sands with interspersions of silty clay lenses. The saturated thickness of the water table aquifer ranges from fifteen to thirty-five feet. Previous studies conducted by Geraghty & Miller, Inc. (1983) indicate ground water flow direction is generally towards the west and north at an average rate of approximately one foot/day.

A total of thirty-nine wells have been installed at the Pedricktown facility (see Figure 8). A Monitoring Well Data Summary is shown in Tables 6, 7 and 8. Thirty-four of these screen the water table aquifer, including twelve pairs of nested wells which screen the upper and lower section of the aquifer. The remaining ten water table aquifer wells screen the water table at various depths with screen lengths ranging from six to thirty feet.

Boring logs indicate that a non-continuous silt/clay layer exists in the southeast section of the site. The silt/clay layer is approximately twenty feet thick as observed in borings RD, HD and CR2 and ranges in depth between five and twenty feet below grade. This layer apparently pinches out somewhere between boring RD and boring BR to the west and somewhere between boring ID and boring 10 to the north. The extent of this layer to the south and east is unknown. Water level elevations in well nests I and R, installed in this area, indicate that perched ground water conditions exist above the silty/clay layer. Previous studies have identified a significant ground water divide in the area of this silty clay layer. While this divide clearly exists for the shallow perched portion of the aquifer, the existence of a divide in the water table aquifer below the silty clay is not defined.

An area of the water table aquifer where data on ground water elevations and geology is limited is the northeast corner of the facility. Subsurface work in this area will provide helpful information on ground water flow direction, contaminant migration, and geologic conditions.

#### Additional Hydrogeologic Studies:

The area to the northeast of the plant proper will require additional monitoring wells to determine the extent of contamination and the direction of ground water flow. A monitoring well will be installed in the northeast area. This well will be installed screening the lower water table aquifer. The well will be installed using auger drilling techniques in accordance with the Overburden Monitoring Well Installation Protocol, to be provided in the Site Operations Plan, and will conform with NJDEP requirements for monitoring well installation. Split-spoon samples will be collected at a minimum of five foot intervals or at changes in stratigraphy. Split spoon samples will be collected in accordance with ASTM Method D-1586-67. The proposed well location is shown on Figure 8, however exact placement will be determined in the field. The well will also be gamma logged following installation.

A site wide ground water sampling program will be instituted following the installation of the above well. Two sampling events are scheduled at a two month interval. These two events will provide the supplemental data base for the evaluation of existing conditions as well as temporal trends.

The following deep water table monitoring and observation wells will be sampled: BR, CR2, 2R2, 3R, 4R, 5R, HD, ID, JD, KD, LD, MD, ND, OD, PD, QD, RD, SD, and the proposed well. Wells will be sampled in accordance with the Ground Water Sampling Protocol, to be provided in the Sampling Plan, and will conform to NJDEP procedures for ground water sampling. The sampling procedure used will include evacuation of a minimum of three well volumes prior to sampling to minimize contact time between ground water and well casing. All deep water table aquifer wells scheduled for sampling will be analyzed for the following parameters:

- ° Antimony\*
- ° Selenium\*

° Gross Alpha & Beta\*

- ° Arsenic\*
- ° pH (field)

\* Total Organic Carbon (TOC)

- ° Cadmium\*
- Conductivity (field)
- \* Total Organic Halogen (TOH)

- ° Chromium\*
- ° Chlorides
- ° Copper\*
- ° Sulfate
- ° Lead\*
- \* Filterable (soluble fraction)/total(1)

The following shallow water table aquifer observation wells will be sampled during the first round of sampling only: HS, KS, MS, NS, and SS. The samples will be analyzed for the same parameters as the deep water table wells.

The additional parameters of filterable/total<sup>(1)</sup> priority pollutant metals and total cyanides will be analyzed on samples from Wells ID, SD and QD in the first round of sampling. These wells were chosen since they appear to be the most impacted wells with respect to ground water quality. Any of the parameters which are identified in any of these three samples within 75% of the Primary Drinking Water Standard will be added to all well samples in subsequent sampling and analysis.

Three wells will be selected based on round one sample results for specific organic hazardous substances analyses during round two. Specific organic substances are defined as priority pollutant organic chemicals.

Ground water samples will be collected and analyzed for filtered gross alpha and gross beta-particle activity during the first round of sampling. If alpha activity levels are found to exceed 5 pCi/L, or are above the area background activity, whichever is higher, then samples will be collected the second round of sampling and analyzed for radium-226. If radium-226 activity exceeds 3 piC/L, then the sample will be analyzed for radium-228. If gross beta particle activity exceeds 50 pCi/L during the first round of sampling, samples will be collected the second round and analyzed for the man-made radionuclides. (1)

Plant and landfill areas will be surveyed with a radiological survey meter to identify possible sources of radiation. The monitoring protocol will be specified in the Site Operations Plan.

If the analytical results of the initial sampling event reveal non-detectable levels of any of the specified parameters in any well, that specific parameter will be deleted from future analysis for that well.

(1) Determination of the utilization of filterable vs. total metals results and specific radiologic analyses is subject to final clarification and agreement within the Site Operations Plan specifications. The MCLs which would be expected to be utilized are based on filterable analysis, except where it pertains to surface water and private water supply wells along State Route No. 130.

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Ground water elevation data will be collected bi-monthly from each well screening the water table aquifer, for a period of six months. This data will be collected and mapped in order to determine the fluctuation of ground water levels and to identify any changes in the direction of localized ground water flow.

#### First Confined Aquifer

#### Previous Studies:

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Beneath the Pedricktown facility the Cap May Formation lies uncomfortably over the Raritan Formation which has been estimated to be approximately 250 feet thick in this area. The deepest on-site well drilled, 8R, penetrated approximately one hundred feet of the Raritan Formation. Two confined aquifer systems were encountered during drilling, referred to here as the first and second confined aquifers. The soil boring samples show the aquifers were comprised primarily of fine to medium light colored sands, interspersed with clays and silts. Separating the aquifers are extensive reddish silty and sandy clay layers. The origin of the Raritan Formation is continental deposition, facies changes are common and lenses present in one well may not be present in a nearby well. These variabilities can occur both vertically and horizontally.

The first confining layer occurs generally at elevations of ten to thirty feet below sea level and ranges in thickness from ten to twenty feet. There are however some areas where this clay unit is not clearly defined from existing logs. Additional work is needed in these areas to determine the integrity of the confining unit. This will be addressed through a detailed review of existing and proposed gamma ray logging of wells previously not gamma logged.

Of the thirty-nine existing wells at the Pedricktown facility three are known to screen the first confined aquifer. These are 9R2, 10 and 11. Water level elevations collected from these wells indicate that ground water flow beneath the site is towards the north and northeast. Previous studies (Geraghty & Miller, Inc., 1983) indicate the average flow rate to be approximately three feet/day. The thickness of this aquifer based on the three boring logs ranges from ten to thirty feet. The outcrop area is believed to be somewhere between the site and the Delaware River.

The direction of ground water flow beneath the Pedricktown facility in the first confined aquifer is most likely controlled by the pumping of the Raritan Formation by nearby industry. If the first confined aquifer is in hydraulic contact with the Delaware River, the ground water quality in that aquifer may be impacted by induced flow of river water into the aquifer. Additional investigatory work is needed in the first confined aquifer to determine actual ground water flow direction, the presence of contaminants and geologic conditions.

#### Additional Hydrogeologic Studies:

An additional first confined aquifer monitoring well will be installed in the area to the northeast of the plant proper (Figure 8). This well will be installed in the northeast area in the immediate vicinity of the water table well proposed for this area. The well will be double cased and installed in accordance with the Double Cased Monitoring Well Installation Protocol for confined aquifers, to be provided in the Site Operations Plan, and will conform to NJDEP requirements for confined aquifer well installation.

All drilling will be conducted by New Jersey State Certified well drillers. At this time it is expected that the wells proposed for the first confined aquifer will be installed using mud rotary drilling techniques.

The proposed well and the three existing first confined aquifer wells (10, 11 9R2) will be included in the two ground water sampling events previously discussed for the water table aquifer. The analytical parameters will also be the same as those listed for the deeper water table aquifer wells. In addition well 11 will be analyzed for cyanide and priority pollutant metals.

During the purging of the newly installed first confined aquifer well for ground water sampling the water levels in the adjacent nested well will be monitored to determine if a direct hydraulic connection exists between the two aquifers. The purpose of this monitoring is to verify that the newly installed first confined aquifer well is properly sealed. If the adjacent water table responds to the purging of the deeper well, it would suggest that the newly installed first confined aquifer well was not successfully sealed and remedial work would be required on that well. Evaluations of any naturally occurring direct hydraulic connection between the water table and the first confined aquifer will be based on the week long ground water elevation monitoring discussed below.

To evaluate the continuity of the confining layer and possible communication between the water table and first confined aquifer ground water levels will be monitored at 15 minute intervals in both aguifers for a period of one week. Previous data collected- (Geraghty & Miller, 1983) indicate that the first confined aquifer is affected by the pumping of nearby industrial wells while the water table aquifer is not. These different responses can be used to evaluate the hydraulic connection between the two aquifers. First confined aquifer wells would be expected to show responses to pumping, however, a nearby direct connection with the water table aquifer would act to dampen the pumping response in the first confined aquifer well. Conversely a water table aquifer well would not be expected to respond to the first confined pumping unless there were a nearby direct hydraulic connection. By monitoring ground water levels in wells in both aquifers across the site the continuity of the confining layer across the site can be evaluated. Two groups of wells will be monitored for one week each. Group I consists of wells 9R2, 10, 11, ID, KD, OD, PD and BR. Group II consists of wells 9R2, 10, new first confined, new water table, 2R2, 4R, LD and MD. This monitoring information will also be utilized to evaluate the effects of nearby pumping of the Raritan Formation on the ground water beneath the Pedricktown facility. Water elevations in the Delaware River during this period will be used to evaluate what, if any, impact tidal fluctuations have on ground water elevations.

#### Second Confined Aquifer

#### Previous Studies:

One well, 8R, screens the second confining unit beneath the Pedricktown facility, screening seven feet of the aquifer. The second confined aquifer is approximately thirty-five feet thick in this area. Boring logs indicate a confining layer with an average thickness of twenty-five feet separates the first and second aquifers. Well 8R shows no evidence of contamination from the Pedricktown facility as indicated in Table 8. The outcrop area for this aquifer is most likely beneath the Delaware River or across the river into Delaware and Pennsylvania. The water level elevation in well 8R has been continually measured at below sea level indicating a component of flow from the Delaware River toward the site in this aquifer system.

Accordingly additional work in the second confined aquifer is not considered necessary or advisable. No evidence of contamination has been detected in this well, and the majority of the wells in the overlying first confined aquifer also are free of contamination. In addition, despite the available drilling techniques the installation of wells through a confining layer risk cross-contamination.

#### Off-Site Wells:

Analytical results for samples collected from the residential wells are presented in Table 9. The sampling and analysis of nine residential water wells located north of the site along State Route No. 130 will occur at the same time as one of the site sampling events. Prior to any sampling, owners of residential and public property identified for study will be contacted and requested to sign a "Hold Harmless" release allowing entry of sampling personnel which may include representatives of NL and/or its designated contractor(s). If problems arise in obtaining access to these locations, the USEPA and NJDEP will be notified immediately to assist in the timely completion of this task.

The construction details of these nine wells, if available, will be reviewed prior to sampling. Where possible, the total depth of each well will be sounded and a water level measurement taken. If water treatment is used, the sample will be collected, where possible, from a pre-treatment location. These well samples will be analyzed for the same parameters discussed above for the deep water table aquifer wells with the exception that samples will not be filtered prior to preservation.

Additional off-site wells may be needed to determine the extent of the ground water contamination plume. The need for these wells cannot be determined until after the first round of ground water quality analyses are completed. In addition, the location of any supplemental wells will be dictated by ground water flow directions and water quality. Consequently, an Interim Report will be prepared at the conclusion of round 1 ground water analyses.

The Interim Report will present information developed during the first round of sampling and analyses as well as other relevant data. It will include recommendations for supplemental wells which may be needed. In addition, it will include a revised sampling and analysis plan to reflect addition of organic priority pollutants at selected wells and deletion of some parameters at others. Also included in the Interim Report will be soil sampling results with recommendations for any desired supplemental analyses. Additionally, initial data and analyses regarding communication between the water table aquifer and the first confined aquifer will be presented.

#### 6.01.5 Air Investigation

Summarize State Implementation Plan for Site:

The NJDEP and the Environmental Protection Agency (EPA) has stated that the Pedricktown facility poses no threat to atmospheric lead contamination based on modeling and field investigations. The following discussion was excerpted from the Federal Register, Vol. 50, No. 37 (February 25, 1985): "National Smelting of New Jersey has permanently ceased operations, and all of the company's operating permits have been revoked by the State. The only remaining source is one of fugitive emissions of dust from open slag storage piles at the abandoned plant site. In its supplemental submittal, the State presented dispersion modeling data which show no predicted violations of the ambient lead standard from the slag pile emissions...EPA finds this demonstration of attainment at the former National Smelting of New Jersey site approvable.

[This source has] been...evaluated by the State according to the following procedure:

A field investigation was conducted to identify fugitive emissions.

The total lead emissions rate was calculated using appropriate fugitive emission factors coupled with control efficiencies and operating parameters, and allowable stack emission rates contained in the source's operating permit.

The source was modeled using EPA-approved dispersion modeling methods."

Accordingly, the site has been determined by the NJDEP and the USEPA to be in compliance with the USEPA's State Implementation Plan requirements for attainment and maintenance of the National Ambient Air Quality Standard for lead. Consequently, no further investigation on the threat of atmospheric lead contamination from the site is necessary.

However, any remedial actions which occur at the site could result in agitation of lead bearing material and the generation of fugitive emissions of dust while these activities occur. Methods for containing and/or reducing fugitive emissions will be addressed in the Feasibility Study. A wind rose diagram for the area is presented as Figure 9.

#### 6.02 Site Investigation Analysis

A thorough summary and analysis of all investigations conducted as part of the Remedial Investigation will be prepared. The objective of this task will be to ensure that the data generated during the Remedial Investigation are sufficient in quality and quantity to conduct the Feasibility Study.

The data will be analyzed and a summary of the type and extent of contamination will be developed. The analysis will include all significant pathways of contaminant migration and a public health and environmental risk assessment conforming to "The Endangerment Assessment Handbook" (prepared for EPA by PRC Environmental Management, Inc., 8/85). The analysis will discuss the degree to which either source control or management of migration remedial actions are

required to mitigate the threat to public health, welfare, or the environment. If the results of the investigation indicate that no threat or potential threat exists, a recommendation to stop the remedial response will be made in a formal report.

The data will also be analyzed relative to the preliminary remedial technologies identified as potentially applicable to the site. Data supporting or rejecting the consideration of types of remedial technologies, compatibility of wastes and construction materials, and other conclusions will be presented.

#### 6.03 Preliminary Remedial Technologies

The data generated by the Remedial Investigation will be used to identify remedial technologies potentially applicable to the problems associated with the site based on technical viability. Preliminary remedial alternatives will then be developed from the identified remedial technologies, pursuant to the NCP.

#### 6.04 Remedial Investigation Report

Following the completion of the field investigation all background and field data will be compiled into a Remedial Investigation Report. Site background information was reviewed during the preparation of this Work Plan. Sections 2 and 3 of the Work Plan summarize the site information necessary to develop and technically evaluate the RI/FS Work Plan. The relatively tight schedule for Work Plan submittal prevented a complete review of NJDEP files and completion of all work associated with the background review. The balance of background information reviewed will be summarized in the Site Operations Plan. As part of the RI Report, a section will be devoted to summarizing site background information. It will also present a discussion of lead smelting and other related processes at the Site and at associated industries in the immediate area. The RI Report will also include a scientific literature search summary. Existing scientific literature regarding lead contamination shall be reviewed and written summaries will be presented regarding:

- The effects of lead on the biota found in the Pedricktown area.
- Secondary lead smelters and other lead sources.
- The significance of environmental lead contamination upon public health, welfare, and the environment based on land usages, demographics, and other relevant human and environmental considerations.
- Ranges of background concentrations of lead in matrices that have been monitored in areas similar to Pedricktown.

The literature search will make use of computerized data base services such as  $Dialog^{TM}$ .

The Report will also include detailed descriptions of the following:

A topographic survey and resultant plot plan including on-site bench marks.

- A summary of all relevant environmental conditions including annual and seasonal climatic changes.
- Geology of the site including soil types and depths, lithology, thickness of unconsolidated deposits, bedrock depth and type using site investigation results or geologic references.
- Plotted results of geophysical work conducted at the site.
- A determination of the areal and vertical extent of contamination.
- Site plan with locations of all wells, test borings and surface water/leachate sampling points.
- Vertical and horizontal variations in groundwater quality.
- Surface water quality and hydrology of the area.
- Types and concentrations of hazardous constituents detected in the surface soil, surface water and groundwater.
- The location and influence of private and public wells on the movement of groundwater.
- The current or potential impacts from the site on the environment and downgradient public and private water supplies.
- Supporting data including: test boring logs, well specifications, field investigation procedures, chemical analyses, in-situ permeability test data, and monitoring well water level elevations.
- A list of remedial programs to be evaluated as part of the feasibility study.
- References to all scientific or technical literature used to prepare the Report.
- Names, titles and disciplines of all professionals engaged in the Report preparation.

#### SECTION 7 - FEASIBILITY STUDY WORK PLAN

#### 7.01 Description of Proposed Response

The first step of the Feasibility Study will be to define the objectives of the remedial action and develop response actions. This involves the identification of site problems and pathways of contamination based on data generated by the Remedial Investigation, and identifying general response actions that address the site problems. The response actions fall into two categories: source control measures and management of migration measures. Once the general response actions are identified, the development and evaluation of alternatives may commence.

#### 7.02 Development of Alternatives

Based on the results of the Remedial Investigation a limited number of alternatives for source control or management of migration remedial actions or both will be developed. Remedial response objectives will be identified as will appropriate remedial technologies. Site-specific remedial response objectives shall be based on public health and environmental concerns, information gathered during the Remedial Investigation, and Section 300.68 of the NCP.

Remedial alternatives will be developed to incorporate remedial technologies, response objectives, and other appropriate considerations into a comprehensive, site-specific approach. The approaches considered will take into consideration State and Federal standards in determining an appropriate degree of cleanup. Alternatives will include non-cleanup and no action options, if appropriate.

The alternatives to be evaluated include, but are not limited to, the following, or combinations of the following control options:

- 1. Establish site security.
- 2. Control surface water impacts by drainage control.
- 3. Control infiltration by installation of a low permeability soil cap or paving.
- 4. Control groundwater movement by providing a groundwater cutoff wall and/or groundwater collection and treatment.
- 5. Selected removal and secure disposal of identified sources of contaminants likely to have an impact and be mobile.
- 6. Sealing any wells within the confined aquifers if evidence exists for contaminant migration along or within the well casing.

In accordance with the NCP, at least one alternative shall be developed in each of the following five categories:

- 1. No action alternative:
- 2. Alternatives that do not attain applicable or relevant and appropriate Federal public health and environmental requirements but will reduce the probability of present or future threat from the site specific indicator(s) and that provide significant protection to public health and welfare and the environment;
- 3. Alternatives that attain applicable or relevant and appropriate Federal public health and environmental requirements;
- 4. Alternatives that exceed applicable or relevant and appropriate Federal public health and environmental requirements; and
- 5. Alternatives for treatment or disposal at an off-site facility.

If Federal public health and environmental requirements are not applicable or relevant and appropriate, a risk assessment shall be conducted to evaluate the risks of the various exposure levels expected to be remaining after implementation of the alternatives.

### 7.03 Screening of Alternatives

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The remedial alternatives identified will be screened to eliminate alternatives that are not feasible or appropriate.

Three broad considerations will be used as a basis for the initial screening: cost; acceptable engineering practices; and effectiveness. More specifically, the following factors must be considered:

- 1. Cost An alternative whose cost far exceeds that of other alternatives without providing substantially greater public health or environmental protection or technical reliability will usually be eliminated from further consideration.
- 2. Acceptable Engineering Practices An alternative that is not feasible for the location and conditions of release, is not applicable to the problem, or is not reliable in addressing the problem will be eliminated from further consideration.
- 3. Effectiveness An alternative that does not protect public health and the environment will not be considered further. If an alternative results in significant adverse impact and provides limited environmental benefits, it will be excluded from further consideration.

### 7.04 Evaluation of Alternatives

Remedial alternatives which pass through the initial screening will be evaluated in greater detail. The alternative evaluation shall consist of a detailed description, cost analysis, environmental assessment and technical evaluation as presented below.

### Detailed Description:

A detailed description of each alternative will be prepared which will address the following issues, as appropriate.

- 1. Description of appropriate treatment and disposal technologies.
- 2. Special engineering considerations required to implement the alternative (e.g., pilot treatment facility, additional studies needed to proceed with final remedial design).
- 3. Operation, maintenance, and monitoring requirements of the remedy.
- 4. Off-site disposal requirements and transportation plans.
- 5. Temporary storage requirements.
- 6. Safety requirements for remedial implementation (including both on-site and off-site health and safety considerations).
- 7. A description of how the alternative could be phased into individual operable units. The description should include a discussion of how various operable units of the total remedy could be implemented individually or in groups, resulting in a significant improvement to the environment or savings in cost.
- 8. A review of any off-site disposal facilities to ensure compliance with applicable Resource Conservation and Recovery Act (RCRA) requirements.

### Cost Analysis:

A detailed cost estimate for each remedial alternative will be developed. The cost of each alternative will be presented as a present worth cost and will include the total cost of implementing the alternative and the annual operation and maintenance cost. A distribution of costs over time will be presented.

### Environmental Assessment:

An environmental assessment for each alternative shall include, at a minimum, an evaluation of each alternative's environmental effects, an analysis of the measure

to mitigate adverse effects, physical or legal constraints, and compliance with the Comprehensive Environmental Response Compensation and Liability Act (CERCLA). Each alternative will be assessed in terms of the extent to which it will mitigate damage to, or protect, public health, welfare, and the environment in comparison to the other remedial alternatives.

### Technical Evaluation:

Each remedial alternative will be evaluated based on technical considerations, including reliability, engineering implementation, and constructability. In terms of reliability, the evaluation shall address operation and maintenance requirements and the demonstrated performance of each component technology. Alternatives which require frequent and/or complex operation and maintenance activities will be considered less reliable than those requiring fewer and/or less complicated operation and maintenance activities. Alternatives which consist of component technologies that have proven effective under conditions similar to those anticipated at the site will be considered more reliable than alternatives with technologies that do not have a proven "track record".

Engineering implementation considers the time required to implement the alternative, safety requirements, and technical practicability. Implementation time includes the time required for additional studies, design, construction, time to realize a benefit, and any other technical considerations required to implement a remedial alternative. An alternative requiring less time to implement would be considered more favorably than an alternative requiring more time. Safety requirements needed to protect on-site workers and offsite receptor populations during implementation are assessed. Those alternatives which pose less of a threat to on-site and off-site personnel, and therefore require less safety precautions to be exercised, will be considered to be more favorable than those requiring more safety precautions. An alternative which is technically more practical to implement based on site conditions will be viewed more favorably than an alternative which is less practical.

### Constructability:

The constructability criterion refers to the ease of construction of an alternative. An alternative which requires exotic construction practices would be considered less constructable than an alternative utilizing standard construction techniques.

The lowest cost alternative that is technologically feasible and reliable and that adequately protects (or mitigates damage to) public health, welfare and the environment will be considered the cost-effective remedial alternative.

### 7.05 Final Report

The Final Report incorporating the Remedial Investigation Report and the results of the Feasibility Study will be prepared and submitted to the EPA. The report will recommend the cost-effective remedial alternative to be implemented for remediation of the site. Relative to the Feasibility Study, the Final Report will contain:

- A summary of all public health and environmental hazards and potential hazards attributable to the site.
- Identification of remedial actions necessary to eliminate existing or potential hazards.
- Identification of technologies capable of achieving the project objectives for each applicable alternative, an evaluation according to the previous section
- Identification of a recommended alternative, including an implementation schedule.

### **Tables**

Table 1

Location	Depth	20M	Dry	Wet
1	0"-2"	2660	2490	2250
	5"-7"	36	33	32
	11"-13"	-	-	-
2	0"-2"	948	853	730
	5"-7"	51	48	43
	11"-13"	-	-	-
3	0"-2"	72	87	81
	5"-7"	-	-	-
	11"-13"	-	-	-
4	0"-2"	47700	36380	32650
	5"-7"	3600	3260	3130
	11"-13"	94	88	84
5	0"-2"	8300	7460	6490
	5"-7"	1260	1140	1090
	11"-13"	123	112	106
£	0"-2"	30400	16400	14500
	5"-7"	83600	58300	54600
	11"-13"	2860	2610	2480
7	0"-2"	2660	2260	2070
	5"-7"	82800	67000	60000
	11"-13"	35700	19400	18400
8	0"-2"	4000	3890	3680
	5"-7"	120	108	102
	11"-13"	-	-	-
Э	0"-2"	612	537	502
	5"-7"	83	73	68 .
	11"-13"	-	-	-
10	0"-2"	6020	4420	4180

### Notes:

- 20 M indicates analytical results for that portion of dried, ground original sample that passed through a 20 mesh standard sieve.
- Dry indicates analytical results for original sample reported on a dry weight basis.
- Wet indicates analytical results for original sample reported on a wet weight basis.
- All values reported in ppm (mg/kg).

- Samples collected by NL Industries, Inc., analysis conducted by Century Environmental Labs, Inc., Thorofare, NJ.

Table 1

Location	Depth	20M	Dry	Wet
	5"-7"	11300	9570	8730
	11"-13"	1543	1300	1230
11	0"-2"	3460	846	793
	5"-7"	15600	13600	12700
	11"-13"	19000	16600	15600
12	0"-2"	10500	9800	7500
	5"-7"	80800	77000	62000
	11"-13"	7940	7600	7300
13	0"-2"	6720	3870	1170
	5"-7"	290	265	185
	11"-13"	-	-	-
14	0"-2"	72	69	62
	5"-7"	-	-	-
	11"-13"	-	-	-
15	0"-2"	836	780.	433
	5"-7"	94	92	69
	11"-13"	-	-	-
16	0"-2"	184	206	184
	5"-7"	-	-	-
	11"-13"	-	-	-
17	0"-2"	86	55	51
	5"-7"	-	-	-
	11"-13"	-	-	-
18	0"-2"	315	198	175
	5"-7"	-	-	-
	11"-13"	-	-	-
19	0"-2"	364	348	294
	5"-7"	-	-	-

- 20 M indicates analytical results for that portion of dried, ground original sample that passed through a 20 mesh standard sieve.
- Dry indicates analytical results for original sample reported on a dry weight basis.
- Wet indicates analytical results for original sample reported on a wet weight basis.
- All values reported in ppm (mg/kg).
- Samples collected by NL Industries, Inc., analysis conducted by Century Environmental Labs, Inc., Thorofare, NJ.

Table 1

Location	Depth	20M	Dry	Wet
	11"-13"	-	-	-
20	0"-2"	172	161	135
	5"-7"	-	-	-
	11"-13"	-	-	-
21	0"-2"	2640	2500	2290
	5"-7"	542	540	509
	11"-13"	216	204	192
22	0"-2"	48	44	41
	5"-7"	-	-	-
	11"-13"	-	-	-
23	0"-2"	36	33	31
	5"-7"	-	-	-
	11"-13"	-	-	-
24	0"-2"	146	141	123
	5"-7"	-	-	-
	11"-13"	-	-	-
25	0"-2"	8800	4620	2190
	5"-7"	160	154	123
	11"-13"	-	-	-
26	0"-2" 5"-7" 11"-13"	3640 10	2890 10 -	2570 9 -
27	0"-2" 5"-7" 11"-13"	142	118 - -	86 - -
28	0"-2"	5140	3890	3540
	5"-7"	2460	2090	1930
	11"-13"	6080	4950	4690

- 20 M indicates analytical results for that portion of dried, ground original sample that passed through a 20 mesh standard sieve.
- Dry indicates analytical results for original sample reported on a dry weight basis.
- Wet indicates analytical results for original sample reported on a wet weight basis.
- All values reported in ppm (mg/kg).
- Samples collected by NL Industries, Inc., analysis conducted by Century Environmental Labs, Inc., Thorofare, NJ.

Table 1

Location	Depth	20M	Dry	Wet
28B	0"-2" 5"-7" 11"-13"	3000 3660	5590 3150 -	5090 2980 -
29	0"-2" 5"-7" 11"-13"	3290 4900 2680	2280 3890 2220	2070 3500 2100
30	0"-2" 5"-7" 11"-13"	142 - -	116 - -	105 - -
31	0"-2" 5"-7" 11"-13"	1820 27	1690 25	1520 24.
32	0"-2" 5"-7" 11"-13"	114 - -	102	91 - -
33	0"-2" 5"-7" 11"-13"	410	324 - -	130 - -
34	0"-2" 5"-7" 11"-13"	710 15	354 9 	333 8 -
35	0"-2" 5"-7" 11"-13"	452 - -	241 - -	220 - -
36	0"-2" 5"-7" 11"-13"	130 - -	53 - -	49 - -

- 20 M indicates analytical results for that portion of dried, ground original sample that passed through a 20 mesh standard sieve.
- Dry indicates analytical results for original sample reported on a dry weight basis.
- Wet indicates analytical results for original sample reported on a wet weight basis.
- All values reported in ppm (mg/kg).
- Samples collected by NL Industries, Inc., analysis conducted by Century Environmental Labs, Inc., Thorofare, NJ.

Table 1

Location	Depth	20M	Dry	Wet
37	0"-2"	800	417	354
	5"-7"	908	598	572
	11"-13"	4680	2800	2640
38	0"-2"	570	314	295
	5"-7"	133	75	72
	11"-13"	-	-	-
39	0"-2"	338	142	135
	5"-7"	-	-	-
	11"-13"	-	-	-
40	0"-2"	4200	3770	2390
	5"-7"	95	85	57
	11"-13"	-	-	-

- 20 M indicates analytical results for that portion of dried, ground original sample that passed through a 20 mesh standard sieve.
- Dry indicates analytical results for original sample reported on a dry weight basis.
- Wet indicates analytical results for original sample reported on a wet weight basis.
- All values reported in ppm (mg/kg).
- Samples collected by NL Industries, Inc., analysis conducted by Century Environmental Labs, Inc., Thorofare, NJ.

TABLE 2
MARSH MUD SAMPLING RESULTS

SAMPLE LOCATION		IALLOW . AMPLE	DEEP SAMPLE						
NUMBER	DEPTH	PB (mg/kg)	DEPTH	PB (mg/kg)					
. 1	12"	20	23 "	1.69					
2	11"	140	21"	25.4					
3	11"	2,600	22"	105					
4	12"	16,800	23 "	243					
5	13"	18,300	26"	2880					
6	13 "·	1,000	26"	897					
7	12"	<b>&lt;</b> 100	24"	85.6					
8	13"	100	62"	7.91					
9	9"	400	18"	8.54					
10	12"	40	23 "	2.68					
11	9"	400	17"	13.8					
12	10"	100	20"	12.8					
13	11"	400	22"	99.8					
14	15"	<100	31"	10.5					
15	12"	200	24" .	17.1					
16	11"	600	21"	55.1					
17	12"	1,000	23 "	660					
18-1	11"	400	21"	133					
18-2	18"	100	37"	16.5					
19	19"	20	38"	3.86					
20	12"	500	24"	65.5					
21	12"	1,000	24"	98.3					
22	11"	1,000	22"	81.0					
23	12"	<1,000	23"	8.88					
23%	12"	<1,000	24"	17.9					
24	11"	<1,000	22"	491					
25	9"	1,400	18"	17.7					
26	11"	<1,000	21"	1.86					
27	12"	<1,000	24"	10.3					
28	12"	1,000	23	4.79					
29	12"	1,000	24"	3.21					
30	9"	400	18"	8.08					
31	11"	300	22"	16.1					
32	12"	250	24"	7.57					
33	10"	150	20"	24.4					
34	12"	100	24"	9.94					

Note: Samples collected by NL Industries, Inc., analyses conducted by Century Environmental Labs, Inc. Thorofare, NJ

### Table 3 Pedricktown RI/FS Amalytical Program

### Analytical Series (2)

Sample Matrix Lab	Sieve <sup>(1)</sup>	Digestion	Filtration	A	B	C	D	E	F	8	H	1	
Soil	140	140	•	140	14	-	-	-	•	-	-	<b>-</b> .	
Slag	3	3	•	3	-	3	-	-	-	-	•	-	
Equipment Residue	10	10	-	10	(3)	-	-	-	-	•	. •	•	
Containerized Solids	25	ක	•	ක	(3)	-	-	-	-	•	-	•	
Containerized Liquids	•	-	-	20	-	-	20	20	-	•	4	<b></b>	
Surface Water												•	
- Round 1 Water	•	-	-	11	1	-	11	-	-	•	-	-	
- Round 2 Water	-	•	-	11	1	-	11	_	-	-	_	-	
- Sediment	-	11.	-	11	1	-	-	-	-	•	-	-	
Marsh Sediment	-	8	•	8	•	-	-	•	-	-	-	-	
Groundwater													
- Water Table Aqui	ifer									<b>/</b> E\			
Round 1	-	-	24	-	-	-	-	-	24	3 <sup>(5)</sup>	-	-,61	
Round 2	•	-	24	-	-	-	-	-	24	-	-	3 <sup>(6)</sup>	
- 1st Confined Aq	uifer .											•	
Round 1	•	· •	4	-	-	-	-	-	4	1	-	-	
Round 2	-	-	4	-	-	-	-	-	4	1	-	-	
- Off-site													
Round 1	•	· <b>-</b>	-	-	-	-	•	-	9	-	-	-	
Round 2	-	-	-	-	-	-	-	-	9	-	-	-	

### Not as

- (1) Lab sieving indicates that soil samples will be sieved through a sixteen mesh stainless steel sieve after drying (8 hrs. at 100 C, or until dry), prior to analysis. Slag samples will be crushed and sieved through a 9.5 mm standard sieve in the laboratory prior to analysis.
- (2) A indicates total lead.
  - B indicates antimony, arsenic, cadmium, chromium, copper, selenium, tin, and zinc.
  - C indicates EP Toxic metals.
  - D indicates pH.
  - E indicates TOC.
  - F indicates antimony, arsenic, cadmium, chromium, copper, lead, selenium, radium, gross alpha and beta, sulfate, chlorides, pH (field), conductivity (field), TOC, and TOH.
  - 6 indicates cyanide and priority pollutant metals.
  - H indicates TOH, gross alpha and gross beta.
  - I indicates priority pollutant organic chemicals.
- (3) Total metal analysis will be conducted on unknown samples.
- (4) Actual numbers of samples will be determined in the field, as discussed in section 6.01.2. For estimating purposes, it is anticipated that 20 samples will be obtained.
- (5) Any of the parameters identified in any of these three samples within 75% of Primary Drinking Water Standards will be added to all well samples in subsequent sampling and analysis.
- (6) As determined from TOC and TOH results of first round sampling.

TABLE 4

ANALYTICAL RESULTS OF ROTARY FURNACE SLAG

NSNJ SITE

PEDRICKTOWN, NJ

Parameter	EP Toxicity Limit	<u>A</u> 1	<u>B</u> <sup>2</sup>	<u>c</u> ²
Arsenic	5.0	0.002	<0.5	<0.2
Barium	100.0	6.3	<0.2	<0.2
Cadmium	1.0	0.03	<0.01	<0.01
Chromium	5.0	0.36	<0.05	<0.05
Lead	5.0	0.14	<0.1	<0.20
Mercury	0.2	0.0002	<0.02	<0.02
Selenium	1.0	0.041	<0.5	<0.3
Silver	5.0	0.01	<0.05	<0.05
Nickel	<del></del>		<0.05	<0.05
Flash Point	60°	60°	-	
Reactive Sulfide		25%	<5.	<5.
Reactive Cyanide		ND	<10.	<10.

- 1. Slag samples analyzed by Century Environmental Testing Labs, Inc. All values given as ppm (mg/l) except Flash Point (°C), reactive sulfide (%) and reactive cyanide (%).
- 2. Slag samples analyzed by Stablex-Reutter, Inc. All values given as ppm (mg/l) except Flash Point (°C), reactive sulfide (mg/g) and reactive cyanide (mg/g).

Table 5

### NSNJ Pedricktown Marsh Water Analytical Results 1981-83

		No: Mai	-		uth rsh	Marsh Discharge			
Parameter	Units	n	Max	n	Мах	ń	Max		
Mercury Hg	mg/l	1	0.0015	. 1	(0.0002	-	-		
Arsenic As	<b>mg/1</b>	2	0.78	1	0.016	1	0.02		
Arsenic Filt.	mg/l	1	0.67	1	0.004	-	-		
Barium Ba	mg/l	16	(0.5	-	-	-	-		
Cadmium Cd	mg/l	18	0.42	1	0.09	i	0.02		
Cadmium Filt.	mg/l	1	0.25	1	0.06	-	-		
Chloride Cl	mg/l	18	335	_	-	-	-		
Iron Fe	mg/l	19	42000	1	6.3	-	_		
Iron Filt.	mg/l	1	4310	1	4.83	-	-		
Lead Pb	mg/l	19	7.52	1	3.08	1	2.75		
Lead Filt.	mg/l	1	1.38	1	0.36	-	-		
Manganese Mn	mg/1	18	11.7	1	0.45	1	1.42		
Manganese Filt.	mg/l	1	10.5	1	0.44	-	-		
Selenium Se	mg/1	18	0.097	1	(0.0005	i	0.013		
Selenium Filt.	mg/l	1	0.113	1	(0.0005	-	-		
Sulfate SO4	mg/1	18	15500	1	246	1.	1400		
Tin Sn	mg/l	1	10.5	_	_	-	_		
T. O. C.	mg/l	1	10	-	-	٠	-		
R. O. D.	mg/l	1	2	-	-	-	-		
C. O. D.	mg/l	15	560	1	49		÷		
Hardness	mg/l (as CaCO3)	1	129	-	-	-	•		
a <del>ll</del>	S. U.	18	5.4	1	4.3	1	4.5		
Phenols	mg/l	1	(0.002	-	•	_	•		
T. D. S.	mg/l	19	23700	1	951	1	7880		
Turbidity	UTN	19	48	1	94	1	83		

- North Marsh indicates the sample was obtained in the vicinity of the north end of the culvert which connects the south marsh and the north marsh.
- South Marsh indicates the sample was obtained in the vicinity of the south end of the culvert which connects the south marsh and the north marsh.
- Marsh Discharge indicates the sample was obtained from the north marsh in the vicinity of well 6.
- If two pH values are recorded, the first is the maximum pH and the second value, in the parenthesis, is the minimum pH.
- Samples collected by ML Industries, Inc., analysis conducted by Century Environmental Labs, Inc., Thorofare, NJ.

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. Table 6 (continued) MSNJ Pedricktown On-site Monitoring Wells
RADIDANALYTICAL DATA

from minimum the tribe to the form

		He	11 1R	Wel	1 2R2		He	11 38	We l	1 4R	Hel	1 SR	•	<b>l</b> e11 6	We	11 BA	iki	1 982	Ue:	1 10	Ve	11 11		He l	1 11R
		Wate	r Table	Water	Table		Water	r Table	Hater	Table	Water	Table	Wate	er Table	2nd A	rtesian	ist A	rtesian	ist A	tesian	ist A	tesian	1 1	st Arl	tesian
Paraweter	Units	, n	Hax	•	Nex		n	Max	n	Nax	n	Kax	n	Nax	n	Max	*	Kax	n	Kax	ħ	Max		n	Ман
Gross Alpha	pCi/l	4	(50.8	6	270	74	5	23.7	5	15. 1	6	23.9	5	33.4	5	155 `	-	-	1	19	ı	100.3	130.8		_
Gross Beta	pCi/l	4	31.6	5	41 6	.1	5	69. 3	5	32.8	6	37.4	5	28.6	5	(14	_	-	-	-	1	12.5	32.8	-	-
Radium (total)	pCi/l	-	-	4	3.05	1.1	-	-	-	-	-	-	-	-	5	2.5 1.2	-	-	-	-	1	2.8 0.	.4	-	
Radium 226	pCi/l	4	11.7	1	0.823	0.25	3	2.1	5	2.11	3	3.06	3	2.19	5	(15	-	-	-	-	-	-		-	-
Ce-144	pCi/l	1	20.5		-		-	(= 38.2	1	(= 48.2	1	(= 41.2	1	47.1	-	-	-	-	-	-	-	•		-	-
Ce-141	pCi/l	1	18.4	•	-		- ,	(= 13.6	1	(* 45.9	1	(= 34.5	1	39.2	-	-	-	-	-	-	-	-		-	-
Ru-103	pCi/l	1	11.4	-	-		-	(= 89.6	- 1	(* 17.2	1	(= 19.2	1	15.3	-	-	_	-	-	-	-	-		-	-
Cs-134	pCi/l	1	2.99	-			-	(* 5.19	1	(= 4.39	1	(= 5.26	1	4.37	-	-	-	-	-	-	-	-		-	-
Ra-226 Di	pCi/l	1	7.20	-	-		-	15 11	1	(* 9.90	1	(= 11.1	1	11.2	-	-	-	•	-	-	•	-		-	-
Ru-106	pCi/l	t	31.1	-	-		-	(* 3.76	1	(= 47.9	1	(= 52.0	1	45.4	-	•	_	-	-	-	-	-		-	-
Cs-137	oCi/}	1	3.35	-	٠.		-	(* 5.76	1	(= 4.78	1	(= 5.46	1	4.98	-	-	-	-	-	•	-	-		-	-
2r-95	pCi/l	1	10.9	-	-		-	(= 11.8	1	(= 18.1	1	(= 18.3	1	16.6	•	-	-	-	-	-	-	-		-	-
Nb-95	pCi/l	1	6.70	-	-		-	(= 6.53	1	(= 12.4	1	(= 10.9	1	11.2	-	-	-	-	-	-	-	-		-	-
Co-58	pCi/l	. 1	5.89	_	-		-	1= 6.22	1	(= II.I	1	(= B. 94	1	10.0		-	- '	-	-	<b>-</b> ,	-	-		-	-
Mn-54	pCi/1	1	3.41	-	-		-	(* 5.24	1	(= 5.59	1	(= 5.53	1	5.90	-	•	-	-	-	-		-		-	-
Fe-59	pCi/1	1	16.3	_	-		-	(= 14.0	i	(= 36. I	1	(= 27.8	1	29.9	_	_	-	-	-	_	•	•		-	-
2n-65	pCi/l	i	6.74	-	-		-	(= 11.1	1	(= 13.1	ì	(= 13.0	1	13.7	-	-	-	_	•	-	-	-		-	-
Co-60	pCi/l	i	2.54	-	-		-	(= 5.38	ì	(* 5.59	1	(= 4, 49	1	5.62	-	-	-	-	-	-	-	-		-	-
K-40	pCi/l	1	36.4	_	_		_	370 90	1	(= 49.6	1	(e 72.1	1	171	-	_	_	-	_	•	-	-		-	-
Cr-51	pCi/l	-	•	-	-		-	-	Ī	(= 268	i	(- 248	-	-	-	_	-	_	-	•	-	_		-	_
1-131	pCi/l	-	-	-	_			-	1		í	•	_	-	-	-	-	-	-	_	_	_		_	_
Ba-140 La	pCi/l	-	-	•	-		-	-	1	(= 312	1	(= 216	-	-	-	•	-	-	-	-	-	•		-	-

Table 6 NSNJ Pedricktown On-site Monitoring Nells Ground Water Quality Data

		ile	21 1R	We11	1 2R2	He	11 38	H	e11 4R	He	11 5R	H	e11 6	He	11 6R	Hei	1 9R2	He	11 10	lite	an n	H	-11 11R
*		Wate	r Table	Water	Table	Vate	r Table	Wate	er Table	Wate	r Table	Vate	r Table	2nd A	Irtesian	ist A	rtesian	ist A	rtesian	ist A	irtesian	ist f	Irtesian
Paraveter	Units	n	Nax	*	Nax	n	Nan	n	Max	n	Нах	n	Max	n	Max	n	Nax	n	Man	n	Max	h	Max
Antimony Sh	mg/1	10	0.146	9	2.3	8	0.178	10		8	0.042	7	0.053	5	10.005	7	10.05	1	0.007	1	0.68	_	•
Arsenic As	#g/1	7	0.07	6	0.156	6	0.036	7	0.035	7	0.021	ı	0.037	5	(0.002	5	10,002	1	(0.002	3	3. 1	1	0.040
Arsenic Filter	mg/1	-	•	•	-	•	•	-	-	-	-	-	•	-	•	-	•	-	•	2	1.9	1	0.040
Barius Ba	eg/l	5	10.5	6	0.1	5	10.5	5	10.5	5	(0.5	51	10.5	5	(0. 1	1	10.1	1	(0. 1	1	(0. 1	-	-
Cadwium Cd	mg/l	7	0.04	6	0.06	6	0.27	7	0.05	7	0.08	53	54	5	(0, 01	8	(0.01	ŧ	(0.01	2	1.77	į	<b>(0,01</b>
Cadwium Filter	mg/1	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	*	-	-	-	•	1	(0. 01
Ditoride Cl	#q/l	8	765	5	500	•	765	7	197		198	22	362	5	23	7	13	1	ස	2	310	•	-
Chromium (Total)		6	0.05	6	0.05	5	0.03	7	(0.05	7	0.02	7	(0.05	5	(0.01	!	(0, 01	1	(0.01	1	0.13	•	-
Chronium (Hex)	øg/1	2	(0. 1	6	10.05	8	<b>(0. 1</b>	2	(0.05	5	(0. 1	2	(0.1	5	(0, 05	i	10.05	1	(0.05	1	(0.05	-	-
Copper	mg/}	4	0.05	6	0.30	5	0.50	4	13.0	3	0.25	5	0.18	5	0. 1	1	0.06	1	(0.01	1	0.12	-	-
Cyanide	mg/}	5	(0.01	6	0.063	2	(0.01	5	(0.01	3	(0.01	2	(0.01	5	0.01		(0.01	ı	(0.01	1	(0.01	-	-
Fluoride	rg/1	4	0.98	6	1.6	5	0.54	5	1.6	5	0.82	5	1.0	3	(0. 1	1	10.1	1	0.1	1	1.9	-	-
Iron Fe	#g/1	7	203	9	1.25	7	27.2		0.49	7	56	23	224	5	0.80	6	3.53	1	1.39	1	87	-	•
Lead Pb	mg/1	16	0.26	14	0.19	16	0.21	16	0.23	14	0.15	31	0.15	10	0.05	13	0.12	5	0.1	7	0.46	1	0.03
Lead Filter	mg/1	5	(0.01	8	0.06	5	0.15	5	(0.01	S	0.02	2	(0.01	5	(0.01	2	(0.01	5	(0.01	•	0.40	1	0.01
Hanganese Mn	<b>99/</b> 1	3	3.7	6	46	3	8.6	3	4	3	1.0	51	3.4	5	0.48	5	0.06	1	0.20	ı	<b>5</b> 5	1	0.82
Marganese Filter		-	*	-	-	•		-	-		-	-	-	-	-	-	-	-	-	-	**	1	0.81
Mercury	wg/1	5	(0.001	7	0.0006	4	0.27	•	(0.001	•	(0.001	4	(0.001	5	0.0005	-	-	!	(0.0005	1	(0,0005	•	•
Nitrate	eg/1	5	13.0	6	3.5	5	91	5	50.3	5	34	5	16.3	5	3.5	1	(1	i	1.2		6.5	-	-
Selenium Se	mq/)	7	0.083	6	0.070	6	0. 034	7	0.072	7	0.091	23	0.051	5	10.005	Ş	(0.005	1	10,005	3	(0.0005	1	0,068
Selenium Filter	eg/1	-	•	-	- -	-	-	-	~	-	-	-	-		-	-	-	•	-		(0.0005	1	0,063
Silver	<b>09/1</b>	6	0.02	6	0.01	5	(0.01	6	0.01	•	(0.01	. 5	(0.01	5	0.04	1	(0.01	j	(0.01	!	(0.01	•	•
Sodium	ag/1	5	2530	6	5080	2	375	2	260	5	248	S	850	5	4.4	1	1.8	1	9	į	3220	:	-
Sulfate SD4	ag/)	4	10000	7	11500	3	<b>8530</b>	4	1270	4	9370	23	12500	6	16	3	3	ı	26	3	14700	1	6670
Salfite	ag/1	-	-	-	-	1	2270	-	-	-	-	-	-	-	-	-	-	-	-	-	-	•	•
Tin Sn	mg/1	•	(0.5	•	0.5		(1.0	•	(1.0	7	(0.5	6	0.5	5 5	(0.1	7	(0.5		10.5	1	(0.5	-	-
Zinc	mg/l	3	0.55	6	0.63	3	1.5	2	0.21	2	1.1	2	0.27		0.03	1	0.04	:	0.20	. !	0.88	-	i
7. O. C.	mg/)	-	-	6	100.8	3	 // AE	-	-	-	- 10.05	1	12.1	5 5	24.5		-	•	3.8		15. 8 40. 05	1	•
Surfactants	mg/1 LRS	5	10.05	6	0.18	-	(0.05 10	5	(0.05	5	10. VS 7	1 7	(0.05 30	5	10.05 A	7	10.05 10		10.05 5		10.05 (5.0	_	-
P. C. D.	mg/1		80	9	42		10 24		15 48	8	-	7	-	5	_	1						-	•
C.O.B.	<b>19</b> /1	•	145	7	210		90	•			76	-	103	3	42	•	21	1	63	. '	160	-	•
Color		1	70 550	•	-	1	5	1	(5		15 350	1	35	5	44	7	***	•	46		- 764	-	-
Hardness O4	mg/1 (as CaCO3) 10N	•	330	7	1300 2	_	1000		617		,500°	5 1	700 2	5	2	- 1	100	:	₹0	:	/81 f	-	-
Odor pH	S.U.	10	2 E/A A1	10	6. B(4. 0)	10	5.4(3.3)	10	5.8(4.5)	10	6.1(4.0)	24	6.1(3.3)		7.5(6.1)	•	7.5(5.1)	•	7.6	3	7.0(4.8)	1	6.6
•		10	5.5(4.4)	9		10		IV	0.072	5		7	0.045	5	0.08	"	7.5(3.1) <b>9.</b> 03	:	10.01	•	(0.01	_	5.0
Phenols T.D.S.	mg/l mg/l	10	0.02 13610	,	0.131 14000	10	0.11 4050	10	1930	9	0. 07 1680	. źs	8630	3	120	•	110	:	146	3	22100	1	- 1795
Terbidity	4LΩ mak, r	10	740	10	95	10	159	10	26	10	16.0	. E3	700		25	9	150	:	1.6	3	40	i	400
Fecal Coliform	Col/ICO al	2	6	2	93	5	0	1	0	2	D	-	-	5	0	i	0	-	-	-	-	:	***
Total Coliforn	Col/100 ml	4	0	4	Ŏ	5	0		0		Ď	-	•	-	-	•	0	•	•	1	0	_	_
Oil & Brease	C017100 W1	•	_	•	-	-	-	•	_	-	-		3	_	_	•	•		_	•	-	_	_
1613 -N		_	_		310	_	_	_	•	_	_	•	-	5	0.13	_	_		0.8	1	825		-
Conductivity	mg/l micromohs	-	-	i	16000	-		_	-		_	-	-	5	153	-	•		138.5	i	15200	1	9350
104			_		145. 6		_	_	_	-		_	_	-	****	_	•	•	1.50. 5	i	228	•	-
Endrin	ug/l ug/l	Ā	(0, 0001	•	173.6	4	(0.0001	4	(0, 0901	4	(0. 0001	-	(0,0001	5	(0. 1	-	-	-	-	:	****	_	-
· Lindane		;	(0.0001	6	(0. 1	i	(0.000)	4	(0.0001		(0.000)	- ;	(0.0001	5	(0.1	_	•	_	_	-	-	_	_
Methoxychlor	ug/l ug/l	ï	(0.001		(1.0	- 1	(0.00)		(0.001		(0.001	7	(0.001	5	(1.0	_	_	_			•	_	_
•		ï	(0.001	6	(1.0	i	10.001	- ;	(0.001	i	(0.001	i	(0.001	5	(1.0	-	-	_	-		-	_	_
Toxephrine	ug/j							7		- 1	(0.001	ï	(0,001	5	(1.0	<u>-</u>	_	_	•		-	-	
2,4-0	ug/1	•	(0.001	_	(1.0	-	(0.001	•	(0.001		<del>-</del>	•		5 5		•	-	-	-	-	-	-	_
. 2.4.5-TP	uq/1	4	(0. 00 j	6	41.0	4	10.001	4	(C. <b>0</b> 01	4	10,001	4	(n. 001	3	(1.0	•	, <del>-</del>	_					

# Table 6 NSNJ Pedricktown On-site Monitoring Wells Ground Water Quality Data

	•	Well	AR	Well	BR	Well	BR2	Well	CR	Well	CR2
		Water	Table	Water	Table	Water	Table	Water	Table	Water	Table
Parameter	Units	n	Max	n	Max	n	Max	n	Мах	n	Max
Antimony Sb	mg/l	1	0.14	-	-	-	-	-	-	-	-
Arsenic As	mg/l	-	-	1	0.003	-	•	1	(0.002	-	-
Arsenic Filter	mg/l	-	•	-	-	-	-	-	-	-	-
Barium Ba	mg/l	55	(0.5	-	-	2	(0.5	20	(0.5	23	(0.5
Cadmium Cd	mg/l	23	1.2	1	0.01	2	(0.01	21	0.02	23	0.04
Cadmium Filter	mg/l	-	•	-	•	-	•	-	•	-	•
Chloride Cl	mg/l	22	4250	-	•	2	13	50	350	23	1500
Chromium (Total)	mā∖]	-	•	-	-	-	•	-	-	•	-
Chromium (Hex)	<b>mg/l</b>	-	-	-	-	-	-	-	-	-	-
Copper	mg/1	-	-	-	•	-	-	-	-	-	- '
Cyanide	mg/l	-	•	-	-	-	•	-	-	-	•
Fluoride	mg/l	-	-	. •	-	-	-	-	-	-	-
Iron Fe	<b>mg/l</b>	55	1200	-	-	2	27.2	20	127	23	1.18
Lead Pb	mg/1	28	1.01	1	0.02	5	0.06	26	0.07	28	0.32
Lead Filter	mg/l	2	0.30	-	-	-	-	2	0.07	5	0.25
Manganese Mn	mg/l	23	16.5	1	4.9	5	0.24	21	3, 56	23	10.8
Mangamese Filter	mg/l	-	-	<b>-</b> .	-	-	•	-	-	-	•
Mercury	mg/l		•	-	-	-	-	-	-	-	-
Nitrate	wā/1	-	-	-	<b>-</b> .	-	•	-	•	-	•
Selenium Se	mg/l	53	0.3	1	(0.005	5	(0.005	21	0.01	23	0.11
Selenium Filter	mg/l	-	-	-	-	-	-	-	•	-	-
Silver	mg/l	-	-	-	-	-	-	-	-	-	-
Sodium	mg/l	-	-	•	-	-	•	-	•	-	•
Sulfate SD4	mg/l	23	20750	1	12500	2	26.4	22	1710	24	19600
Sulfite	mg/l	-	-	-	-	-	-	-	•	-	-
Tin Sn	mg/l	-	-	-	-	-	•	-	-	•	-
Zinc	mg/l	-	-	-	-	-	-	-	-	-	-
T.O.C.	mg/l	2	27	-	-	•	-	-	-	5	13.8
Surfactants	mg/1 LAS	-	•	-	-	-	-	-	-	-	-
B. C. D.	mg/l	-	•	-	-	-	-	-	-	-	-
C. D. D.	mg/l	-	-	-	-	-	-	-	-	-	•
Color		-	-	-	-	-	-	-	-	-	•
Hardriess	mp/l (as CaCO3)	-	-	-	-	-	-	-	-	-	-
Odor	TON	-	-	-	-	-	-	-	-	-	-
버	S. U.	23	4.0(1.7)	2	5.9(4.9)	2	6.1(5.9)	21	7.3(5.7)	23	6.5(4.8)
Phenols	mg/l	-	-	-	-	-	-	-	-	-	-
T. D. S.	mg/l	24	26760	1	16700	2	114	22	1410	24	30100
Turbidity	NTU	22	20	1	2.8	2	89	20	125	55	22
Fecal Coliform	Col/100 ml	-	-	-	-	-	- ,	-	-	•	-
Total Coliform	Col/100 ml	•	-	-	-	-	-	-	-	-	-
Dil & Grease		•	-	-	-	. •	-	-	-	-	•
NH3 -N	mo/l	-	•	-	-	-	-	-	-	• ,	-
Conductivity	micromohs	-	-	-	-	-	-	-	-	-	-
TOH	ו/מַט	~	-	-	-	~	-	-	-	-	-
Endrin	u <u>n</u> /1	-	-	-		-	-	-	-	-	-
_indane	ug/l	-	-	-	-	-	•	-	•	-	-
Methoxychlor	ug/l	-	-	-	-	-	-	-	-	-	-
Toxaphrine	up/l	-	-	-	-	-	-	-	- '	-	-
2, 4-D	ug/l	-	-	-	-	-	-	-	-	-	-
2,4,5-TP	ug/l	-	-	-	-	-	-	-		-	

# Table 6 (continued) NSNJ Pedricktown On-site Monitoring Wells RADIDANALYTICAL DATA

			Well	AR	Well	BR	Well	BR2	Well	CR	Well	CR2
			Water	Table	Water	Table	Water	Table	Water	Table	Water	Table
: ·	Parameter	Units	n	Max	n	Max	n	Max	n	Max	<b>Y</b> i	Мах
	Gross Alpha	pCi/l	_	-	-	_	-	-	_	_		-
	Gross Beta	pCi/l	-	-	-	-	-	_	-	_	_	-
	Radium (total)	pCi/l	-	-	-	-	-	-	-	-	_	-
	Radium 226	pCi/l	•	-	-	-	-	-	-	-	-	-
	Ce-144	pCi/l	•	-	-	_	-	-	-	-	_	_
	Ce-141	pCi/l	-	-	-	-	-	-	_	-	-	
	Ru-103	pCi/l	-	-	-	-	-	-	-	-	-	_
1	Cs-134	pCi/l	-	-	_	-	-	-	-	-	-	_
	Ra-226 Bi	pCi/l	-	-	-	-	-	-	_	-	-	_
	Ru-106	pCi/l	•	-	-	•	-	-	-	-	_	_
	Cs-137	pCi/l	-	-	-	-	-	-	-	•	-	_
	Zr-95	pCi/l	-	-	-	-	-	-	-	-	•	-
	Nb-95	pCi/l	•	-	-	-	-	-	-	-		-
•	Co-58	pCi/l	-	-	-	- ,	-	-	-	-	-	-
	Mn-54	pCi/l	-	-	-	-	-	-	-	-	-	-
	Fe-53	pCi/l	-	-	•	-	-	-	-	-	-	-
	Zn-65	pC:/1	-	-	-	- '	-	-	-		-	-
3:	Co-50	pCi/l	-	-	-	-	-	-	-	- '	•	-
	K-40	pCi/l	-	-	-	-	-	-	-	-	-	-
	Cr-51	pCi/l	-	-	-	-	-	-	-	-	-	•
	I-131	pCi/l	-	-	-	-	-	-	-	-	-	-
	Ba-140 La	pCi/l	-	-	-	-	-	-	-	-	-	-

### \* Notes:

<sup>-</sup> If two pH values are recorded, the first value is the maximum pH and the second value, in the parenthesis, is the minimum oH.

<sup>-</sup> Samples collected by ML Industries, Inc., Geraghty & Miller, Inc., and New Jersey Department of Environmental Protection. Analyses conducted by Century Environmental Labs, Inc., and New Jersey Department of Environmental Protection.

Table 7 NSNJ Pedricktown Observation Hells Ground Nater Duality Bata

		He1	l HS	Well	110	Wej)	15	We11	10	We11	JS	Well	OL.	Hel I	KS .	We11	I KD	Wel !	LS	Well	LD	He11	MS	Ue I I	190
		Water	r Table	Hater	Table	Water	Table	Hater	Table	Water	Table	Hater	Table	Hater	Table	Hater	Table	Water	Table	Water	Table	Water	Table	Water	Table
Parameter	Units	n	Max		Nax	n	Мак	R	Мах	n	Non	n	Max	n	Nan	n	Max	n	Max	n	Max	n	Max F	n	Max
Antimony Sb	<b>ug/</b> ]	-	•	1	10.05	1	(0.05	1	(0, 05	-	-	-	-	1	(0.05	-	-	-	-	-	-	-	•	-	-
Arsenic As	mg/1	-	-	1	0.024	1	0.041	1	0.232	-	-	-	•	1	0.046	-	-	-	•	-	-	-	_	-	-
Arsenic Filter	mg/l	-	-	-	•	-	-	-	•	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	_
Barium Ba	og/l	-	-	-	•	-	-	-	•	-	-	-	-	-	-	-		•	-	-	-	-		-	
Cadmium Cd	wg/L	-	-	1	0.06	1	0.02	1	0.02	-	•	•	-	1	0.27	-	-	•	-	•	•	-	-	-	-
Cadwium Filter	mg/1	-	-	-	-	-	-	-	-	-	-	-	-	-	•	-	•	-	-	-	-	-	-	•	-
Chloride Cl	mg/l	-	-	-	•	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	•	-	•		-
Chromium (Total)	<b>ag/1</b>	-	-	1	0. 16	1	(0.05	1	0.29	-	-	-	•	1	0.22	-	-	•	-	-	-	-	_	•	_
Chronium (Hex)	mg/1	-	•	-	•	-	-	-	-	-	-	-	-	-	- A 47	_	_	_	_	_	_		_	_	_
Copper	mg/i	-	-	ı	0.13	1	(0.05	1	0.14	•	•	•		1	0.43	_	_	_	_	_	_	_	_	_	_
Cyanide	mg/l	-	-	-	-	-	-	-	-	- •	•	•	-•	-	-	_	_	_	_	_		_	_	_	-
Fluoride	mg/1	-	-	-	-	•	-	•	-	-	<del>-</del>	-	-	-	-	_		_	_	_	_	_	_	_	_
Iron Fe	mg/]	-	-	-		-		:		-	10.73	5	0.21	3	0.39	2	0.31	2	0.17	2	0.32	2	0.24	2	3.86
Lead Pb	mg/l	Ş	2.65	. 3	0.18	3	1.10	3	0.23 (0.01	_	9.59		0.02	3	0.23	3	0.28	3	0.06	_	0.32	3	0.24	3	3.62
Lead Filter	mg/]	3	<b>2.3</b> 6	3	0.06	3	0.85	3	10.VI	3	7. 37	-	v. uz	-	V. C.3	-	V. EU	-	-	_	*	•	-	-	-
Mangarese No	Mg/1	-		•	_	-	-	-	_	_	_	_	_		_	_		_	-	_		_			
Manganese Filter		-	-	1	(0,001	ī	(0,001	ī	0.001	-	_	_	_	1	(0.001	_	_	_	_	_	-	_	_		_
Mercury	mg/1	-	•		10.001		10.001	-	0.001	_	_	-	-	•	10.001	_		_	-	-	_	_	-	-	-
Mitrate	mg/l .	-	-	ī	0.104	1	0.081	•	0.14	_	-		_	1	0, 157	_	_	-	-		-	_	-	_	-
Selenium Se	mg/1		-	•	0.107	•	- V. VOI	•	-	_	_	_	_	:	-	-	_		-	-		_	-	_	_
Selenium Filter	mg/1	•	-	1	0.01	1	(0.01	1	0.052	_	_	_		1	0.03	_		-	_		•	_	•	_	-
Silver	mg/1 \	-	-		U. VI	•	10.01	•	V. V.A.	_	_	_	_	:	-		•	_	_	_	_	-	_	-	-
Sedium Solfate SD4	mg/l mg/l	3	7500	3	2580	3	1250	3	483	3	4490	3	1360	5	3520	3	1480	3	76	3	11100	3	8550	3	212
Sulfite	mg/1 mg/1	-	-	•	-	-	-	-	-	_	•	-	_	_		_	-	-	-	-	-	-	-	-	-
Tin Sn	mg/1 mg/1	-	_	-		_	-	-	-	_	-	-	-	-	-	-	-	-	-	-	•	-	-	-	-
line	mg/1	_		1	0.01	1	0.20	1	0.33	_	_	-	-	1	1.4	-	-	-	-	-	-	-	-	-	-
T.O.C.	eg/l	_	-	:	-	_	-	_	-	-		-	-	-	-	-	-	-	-	-	-	-	-	-	-
Surfactants	mg/1 LAS	-		_	-	_	_	_	-	-	-	-	-	-	-	-	-	-	•	-		-	•	-	-
B. D. D.	mg/1 C/G	-	_	_	-	-	-	-	_	-	_	-	-	-	-	-	-	-	-	-	-	-	-	-	-
C. O. D.	mg/l	-	-	-	-	-	_	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	•
Color	y	_	_	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Hardness	mg/1 (as CaCO3)	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	•	-	-	-	-	-	-
Odor	TON	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	•	•	-	-	•	-	-	-	-
He	S.U.	3	4,612.6	3	5.8(4.2)	3	6.0(4.5)	3	6.3(4.0)	3	5.0(3.4)	3	6.1(3.5)	3	5.1(3,3)	3	6.0(4.4)	3	5.0(3.3)	3	4.4(2.6)	3	4.812.4)	3	7.3(4.0)
Phenois	m/1	-	-	-	-	-	<b>-</b> .	-	-	-	-	-	-	-	-	•	• ,	-	-	-	-	•	-	-	-
1. D. S.	<b>89/3</b>	8	8250	2	3500	2	1753	2	855	2	5600	8	2360 ,	2	5320	5	2160	5	<b>272</b>	5	15800	5	11300	5	373
Terbidity	MTU	2	1000	2	5.5	2	62	2	250	2	3.8	2	4.5	2	45	2	30	5	73	5	11000	8	16	2	70
Fecal Coliform	Col/100 ml	-	-	-	-	-	-	-	-	-	-		-	-	•	-	-	-	-	-		-	•	-	-
Total Coliform	Col/100 el	-	-	-	-	-	•	-	-	-	-		-	-	-	-	-	-	-	-	•	-	-	-	•
Dil & Grease		-	-	-	-	-	-	-		-	•	+ '	-	-	-	-	-	-	-	-	-	-	-	-	-
NH3 -N	09/1	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	•	-	•
Conductivity	nicrosohs	-	-	-	-	-	-	-	-	-	-	-	•	-	₹, .	-	-	-	-	-	•	-	-	-	•
TOH	ug/i	-	-	-	-	-	-	-	-	-	-	-	•	-	-	-	•	-	-	-	-	-	-	•	-
Endrin	ug/l	-	-	-	-	-	-	-	•	-	-	-	-	-	-	-	-	•	-	•	-	-	•	-	•
Lindare	ug/l	-	-	-	•	-	-	-	-	-	-	-	-	-	-	-	-	-	-	•	• .	-	•	-	•
Methoxychlor	ug/l	-	-	-	-	-	-	•	-	-	•	-	•	-	•	,	•	•	-	•	-	-	•	-	-
Toxaphrine	<b>ug/1</b>	•	•	-	•	-	-	-	•	-	•	-	-	•	-	-	-	-	•	-	<del>-</del>	•	•	-	_
2,4-D	wg/l	•	•	-	•	-	•	-	-	-	-	-	•	-	-	-	•	-	•	•	•	•	•	•	-
2,4,5-1P	<b>uq/</b> }	-	•	-	•	-	•	-	-	-	-	-	•	-	-	-	-								

医生物 化物料 化邻苯苯酚 医乳腺 经未经 医二氏病 化电池

•		Well	1 HS	Well	1 HD	Heli	I IS	· Well	110	He l	l JS	lle	l JD	He	11 KS	Well	KO)	Well	LS	Hell	LD	He11	MS	Wel	1 10
		Water	r Table	Water	r Table	Water	r Table	Water	r Table	Water	Table	Wate	r Table	Wat	er Table	Water	Table	Water	Table	Hater	Table	Water	Table	Water	r Table
Parameter	Units		Max	•	Max	n	Max	n	Max	n	Max	n '	Max	n	Max	n	Max	n	Max	R	Max	n	Мак	n	Plan
Bross Alpha	pCi/l	•	-	-	-	-	-	-	-	1	193 40	- (	133 26			-	-	-	•	-	-	1	15.0 3.12	2 1	51 16
Gross Beta	pCi/I	-	, <del>-</del>	-	-	-	-	-	-	1	166 25	1	96 16	-	-	-	-	-	-	-	-	1	19.6 2.19	1	55 10
Radium (total)	pCi/l	•	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Radium 226	pCi/l	-	-	-	-	-	-	-	-	-	-	-	-	_	-	_	-	-	-	-	-	_	-	-	-
Ce-144	pC1/1	-	-	-	-	-	-	-	-	1	39.0	1	22.5	-	-	-	-	-	-	•	-	1	(= 40.6	1	31.3
Ce-141	pCi/I	-	-	-	-	-	-	-	-	1	30.1	1	5.43	-	-	-	-	-	-	•	-	1	(= 35.8	1	26.3
Ru-103	pCi/l	-	-	-	-	-	-	-	-	1	16.5	1	10.6	-	-	-	-	-	-	-	_	1	(= 13.7	1	10.4
Cs-134	pCi/l	-	-	-	-	-	-	-	-	1	4.75	1	3.57	-	-	- `	-	-	-	-	-	1	(= 3.19	1	2.98
Na-226 Di	pCi/l	•	•	_	-	-	-	-	-	1	16 9	1	8.41	-	-	-	-	-	•	-	-	1	(= 9.08	1	8.08
Re-106	pCi/l	-	-	-	-	-	-	-	-	1	47.8	1	35.2	-	-	-	-	-	-	-	-	1	(= 40,6	1	30.2
Cs-137	pCi/l	-	-	-	-	-	•	-	-	1	5.10	ı	3.91	-		-	-		-	-	-	1	(= 4.08	Ī	3.18
Zr-95	pC1/1	-	-	-	-	_	-	-	-	1	16.4	1	12.1	-	-	-	-	•	-	-	-	1	(= 15.0	1	11.1
Mb-95	pCi/l	-	-	-	-	-	-	-	-	1	9.30	1	7.00	-	-	- '	-	•	•	-	_	1	(= 10.5	ì	7.73
Co-58	pCi/I	_	-	-	•	-	-	-	-	1	8.71	1	6. 12	-	-	-	-	-		-	-		(= 6.72	1	6.63
Mn-54	pCi/l	-	-	_	-	-	-	-	-	1	5.30	1	3.83	_	-	-	_	-	-	-	-		(= 5.13	1	3.97
Fe-59	pCi/l		-	-	-	-	-	-	-	1	25.4	1	17.1	-	-	-	-	-	-	_	-		(= 27.7	i	20.1
2n-65	pCi/l		-	-	-	_	-	-	_	1	12.0	1	8.17	-	-		-	-	-	-	-		12.2	i	9, 14
Co-60	pCi/l	_	-	-	-	-	-	-	-	1	5.02	1	4.05	-	-	_	-	-	-	-	_		(= 5.09	i	3.79
K-40	pCi/l	_	_	-	-	-	-	_	-	1	50.9	1	43.6	_	-	-	-	-	_	-	-		(= 48.1	i	40.3
Or-51	pCi/l	-	-	-	-	-	-	-	-	-	-	_	•	-		_	-	-	-	-	-		(= 209	-	-
1-131	pCi/l	-	-	-	-		-	_	-	-	-	_	-	-	•	-	_		-	•		-		_	
Ba-140 La	pCi/l	-		-	_	-	-	_	_	-	-	-		-	-	-	-	-	_	-		-	(= 233	-	-

10

• .	-	-	-	-	-	-	-	-	-	-	1	(0.05
-	-	-	•	-	-	-	-	-	-	-	1	0.031
-	-	-	-	-	•	•	-	-	~	-	-	-
-	-	-	-	_	-	-	-	-	-	-	-	-
-	-	•	-	-	-	-	-	-	-	-	1	0.01
•	-	-	-	-	-	-	-	-	-	-	-	-
-	-	-	-	-	-	•	•	-	-	-	-	-
-	-	-	-	•	-	-	•	-	1	0.4	1	0.29
-	-	-	-	-	-	-	-	-	•	-	-	-
-	-	-	-	-	-	-	-	-	1	0.1	1	0.28
•	-	•	-	-	-	-	-	-	-	-	-	-
-	-	-	-	•	-	-	-	<u>- ·                                     </u>	-	•	-	-
-	-	-	-	-	-	-	-	-	-	-	-	-
0.27	1	0.08	2	0.20	5	2.96	2	2.01	2	2.7	2	0.32
0.14	2	0.08	2	0.14	3	2,52	3	1.70	2	(0.01	5	0.05

Well RS

Well RD

Well SS

Well 50

**Water Table** Water Table Water Table Water Table Water Table Nater Table Water Table Water Table Water Table **Water Table Water Table Water Table** Max Max Parameter Units Antimony Sb **eg/1** Arsenic As mg/1 Arsenic Filter mg/l Barium Ba mg/1 Cadmium Cd **eq/1** Cadmium Filter mp/1 Chloride Cl 00/1 Chromium (Total) mq/1 Chronium (Hex) **eg/1** Copper mg/1 Cyanide mg/l Fluoride 09/1 iron Fe mg/1 2 2 2 2 0.02 Lead Pb mg/1 0. 12 3 0.54 3 0.02 Lead Filter mq/l 0.01 Hanganese Mn **1/g** Manganese Filter **29/1** 10.001 **mg/**1 Hercury Nitrate eg/i 0.044 Selenius Se m/1 Selenius Filter mg/1 0.04 Silver **m/1** Sedium mg/1 Sulfate SO4 mg/1 Selfite **ag/**] Tin Sn **mq/1** 2.0 0.91 Zinc mg/1 T.O.C. mg/1 Serfactants eg/1 LAS D. O. D. mg/1 C.O.D. aig/1 Color mg/1 tas CaCD3) Hardress Odor TON 2 7.2(5.0) 3 S.U. 5.5(3.5) 3 Phenois m/1 215 2 35100 2 2 5 2400 T. D. S. **m/**] \$ 550 2 3030 11000 150 125 150 55 Turbidity NTU 757 2 33 2 95 2 20 2 5 5.0 62 8 Fecal Coliform Col/100 ml Total Coliform Col/100 ml Dil & Grease M13 -N **mg/**} Conductivity **sicrosohs** TOH 49/1 Endrin ug/1 Lindane wg/1 Hethoxychlor ug/l Toxaphrine wg/1 2.4-D **ug/**1 2,4.5-TP **uq/!** 

Table 7 NSNJ Pedricktown Observation Hells Bround Water Quality Data

Well OS

Mel1 00

Well PD

Well PS

H 0 0 ັກ Hell NS

He11 NO

Well 05

Hell 00

Table 7 (continued) NSNJ Pedricktown Observation Hells RADIDANALYTICAL DATA

		Well	NS.	He11	ND	Well	05	He ?	1 00	Wel	1 PS	Well	PO	We31	05	Hell	<b>QD</b>	lle 1	ns es	He?	RD RD	He1	SS	Wel	1 50
		Water	Table	Water	Table	Water	Table	Water	r Table	Vater	r Table	Water	Table	Water	- Table	Water	Table	Water	r Table	Water	r Table	Water	Table	Hate	r Table
Parameter	Units		Max	n	Max	n	Max	•	Ман	n	Max	'n	Nax	n	Nax	n	Max	n	Nex	n	Max	n	Nan	n	Nax
Gross Alpha	pCi/l	_	-	-	_	-	-	-	-	-	-	•	-	-	-	-	-	1	604 90	1	76 16	_	-	-	-
Gross Beta	pCi/l	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1	350 34	1	69 15	-		-	-
Radium (total)	pCi/l	-	-	-	-	-	-	-	-	-	•	-	-	-	•	-	-	-	-	-	-	-	-	-	-
Radium 226	pCi/l	-	-	-	-	-	-	-	-	-	-	-	-	-		-	-	-	-	-	-	-	-	-	-
Ce-144	pCi/l	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1	20.5	-	-	-	-
Ce-141	pCi/l	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	•	•	1	16. 3	-	-	-	-
Ru-103	pCi/l .	-	-	-	-	-	-	-	-		-	-	-	-	-	-	-	-	-	1	9. 70	-	-	-	-
Cs-134	pCi/l	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	•	-	1	2.69	-	-	-	-
Ro-226 Di	pCi/l	•	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1	6.53	-	•	-	-
My-106	pCi/i	-	•	-	-	-	-	-	-	-	-	-	÷	-	-	-	-	-	-	i	31.1	-	-	-	-
Cs-137	pCi/l	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	•	•	-	1	3.28	-	-	-	-
Zr-95	pCi/l	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1	10.7	-	-	-	-
Nb-95	pCi/l	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1	2.72	-	-	-	-
Co-58	pCi/l	•	-	-	-	-	-	-	-	-	-	-	-	- •	-	-	•	•	-	1	5. 33	-	-	-	-
Mn-54	pCi/l	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	•	1	3.30	-	-	-	-
Fe-59	pCi/l	-	-	-	•	-	-	-	-	-	-	••	-	•	-	-	-	-	-	ŧ	15.0	-	•	•	-
Zn-65	pCi/l	-	-	•	-	-	-	-	-	-	-	-	-	-	-	-	-	-	•	1	7.01	-	-	-	-
Co-60	pCi/l	-	•	•	-	-	-	-	-	-	-	-	-	-	-	-	•	•	-	1	3.00	-	-	-	-
K-40	pCi/1	+	-	-	•	-	-	•	-	-	-	-	-	-	-	-	-	-	-	1	30.7	-	-	-	-
Cr-51	pCi/l	-	-	-	-	-	-	-	-	-	-	-	-	-	•	-	-	-	-	-	-	-	-	-	-
1-131	pCi/l	•	-	-	-	-	-	-	-	-	-	-	-	-	-	•	-	-	-	-	-	-	-	-	-
Ba-140 La	pCi/l	•	-	-	•	-	-	-	-		-	-	-	-	-	-	-	-	-	•	-	-	-	-	-

# Table 7 NSNJ Pedricktown Observation Wells Ground Water Quality Data

		, Well	T2	Well	T2-1	Well	T2-2	Well	T2-3	Well	T4
		Water	Table	Water	Table	Water	Table	Water	Table	Water	Table
Parameter	Units	'n	Max	n	Max	n	Max	n	Max	n	Max
Antimony Sb	mg/l	-	-	-	-	-	-	-	-	-	•
Arsenic As	mg/l	-	•	-	-	-	-	-	-	-	-
Arsenic Filter	mg/l	-	-	-	-	-	-	-	-	-	-
Barium Ba	mg/1	-	•	-	-	***	-	-	-	-	•
Cadmium Cd	mū/l	-	-	•	•	-	-	-	•	-	•
Cadmium Filter	mg/l	•	-	-	-	-	-	-	-	-	<b>-</b> .
Chloride Cl	mg/l	-	•	-	-	-	-	-	-	-	-
Chromium (Total)	mg/l	-	•	-	-	-	-	-	-	-	-
Chromium (Hex)	mg/l	-	-	•	-	-	-	-	-	. •	-
Copper	mg/1	-	-	-	-	-	-	-	-	-	-
Cyanide	<b>w</b> g/l	~	-	-	-	-	-	-	-	-	•
Fluoride	mg/l	-	•	-	-	-	-	•	-	-	-
Iron Fe	mg/l	-	-	-	-		-	-	•	-	-
Lead Pb	mg/l	1	1.38	1	1.99	1	1.89	1	0.40	2	1.11
Lead Filter	mp/l	1	1.25	1	1.67	1	1.00	1	0.40	5	0.77
Manganese Mn	mg/l		-	-	-	-	-	-	-	-	<b>-</b> .
Mangamese Filter	-	-	-	-	-	-	-	-	-	-	-
Mercury	mg/l	-	-	_	-	•	-		-	-	-
Nitrate	mg/l	-	-		-	-	-	-	•	-	-
Selenium Se	mg/l	-	-	-	-	-	-	-	-	-	• .
Selenium Filter	mg/l	-	-	-	-	-	-	-	-	-	-
Silver	mg/l	-	•	-	-	-	-	-	-	-	_
Sodium	#g/l		8150	- 1	9100	1	4200	-	14000	5	11500
Sulfate 504 Sulfite	mg/l	1 -	- 120	1	3100	1	4200	i -	-	-	11200
Tin Sn	mp/l	-	_	_	-	-	_	_	_	_	_
Zine	mg/l mg/l	_	-	_	-	_	_	_	-	-	_
T. O. C.	mg/l	_	-		-	_	_	-	-	-	•
Surfactants	mg/1 mg/1 LAS	_	•	_	-	_	_	-	_	-	_
B. C. D.	mg/l	_	_	_	-	_	_	-	_	-	_
C. O. D.	#g/l	_	•	-	_	-	_	_	_	•	_
Color	-8, +	_	-	•	_	-	_	_	-	-	-
Hardness	mg/l (as CaCO3)	-	•	-	_	_	-	-	_	_	•
Odor	TON	-	-	-	_	_	_	-	-	-	-
pΗ	S. U.	2	5.2(3.9)	1	3.8	1	3.8	1	2.8	2	5.6(3.9)
Phenols	mg/1	-	-	-	-	-	-	-	-	_	-
T. D. S.	mg/l	2	11200	_	_	-	-	-	-	1	10700
Turbidity	אדט	1	66	-	-	_	_	-	-	1	70
Fecal Coliform	Col/100 ml	-	-	-	-	-	-	_	-	_	-
Total Coliform	Col/100 ml	-	_	-	•	-	_	-	-	-	-
Dil & Brease	-	_	•	_	<b>-</b> ·	-	-	-	-	-	-
NH3 -N	mg/1	-	-	-	-	-	-	-	_	-	-
Conductivity	micromohs	-	-	-	-	_	-	-	-	-	-
TOH	ug/l	-	-	-	_	-	-	-	-	-	-
Endrin	ug/l	-	•	_	-	-	-	-	-	-	•
Lindane	ug/l	-	-	-	-	_	-	-	•	•	-
Methoxychlor	ug/l	-	-	-	-	-	-	-	-	-	-
Toxaphrine	ug/l	-	-	-	-	-	-	•	-	-	-
2,4-D	ug/l	-	-	-	-	-	-	-	-	-	-
2, 4, 5-TP	ug/l	-	•	• ,	-	-	-		-	-	-
		-									

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# Table 7 (continued) NSNJ Pedricktown Observation Wells RADIDANALYTICAL DATA

		Well	T2	Well	T2-1	Well	72-2	Well	T2-3	Well	74
		Water	Table	Water	Table	Water	Table	Water	Table	Water	Table
Parameter	Units	n	Ħax	n	Max	'n	Max	n	Max	<b>Y</b> i	Max
Gross Alpha	pCi/l	-	-	-	-	-	<b>-</b> ,	-	- ,	-	-
Gross Beta	pCi/l	-	-	-	-		-	-	-	-	-
Radium (total)	pCi/l	-	-	-	-	`-	-	-	-	-	-
Radium 226	pCi/l	-	-	-	-	-	-	-	-	-	-
Ce-144	pCi/l	-	-	-	-	-	-	-	-	-	-
Ce-141	pCi/l	-	-	-	-	-	-	-	-	-	-
Ru-103	pCi/l	-	-	-	-	-	-	-	-	-	-
Cs-134	pCi/l	-	-	-	-	-	-	-	-	-	-
Ra-226 Bi	pCi/l	-	-	-	-	-	-	-		-	-
Ru-105	pCi/l	-	-	-	-	-	-	-	-	•	-
Cs-137	aCi/l	-	-	-	-	-	-	-	-	-	-
2r-95	pCi/l	-	-	-	-	-	-	-	-	-	-
Nb-95	pCi/l	-	-	-	-	-	-	-	-	-	-
Co-58	pCi/l	-	-	-	-	-	-	-	-	-	-
Mn-54	pCi/l	-	-	-	-	-	-	-	-	-	-
Fe-59	pCi/l	-	-	-	-	-	-	-	-	-	-
Z <del>n-6</del> 5	pCi/l	-	-	-	-	-	-	-	-	-	-
Co-60	pCi/l	-	-	-	-	-	-	-	-	-	-
K-40	pCi/l	-	-	-	-	-	-	-	-	-	-
Cr-51	pCi/l	-	-		-	-	-	-	-	-	-
I-131	pCi/l	-	-	-	-	-	-	-	•	-	-
Ba-140 La	nCi/1	-	-	_		- '	-	-	-	_	_

<sup>-</sup> If two pH values are recorded, the first value is the maximum pH and the second value, in parentheses, is the minimum pH.

<sup>-</sup> Samples collected by NL Industries, Inc., Geraghty & Miller, Inc., and New Jersey Department of Environmental Protection; analysis conducted by Century Environmental Labs, Inc. Thorofare, NJ., and New Jersey Department of Environmental Protection.

Table 8 "NSNJ Pedricktown Bround Water Abatement System Bround Water Duality Data

		GMUS	S MAIL	emus	S MAZ	GMRS	S NH3	GHR	S NEI	GHAS	NES	GMAS	NE3	8MRS	S SNI	GHAS	SMS I	SWPS !	SE SPLIT	Britis	SEI	enus	SES.	GHA	S <b>9</b> E3
		Water	Table	Water	r Table	Water	Table	Hate	r Table	Water	Table	Water	Table	Water	- Table	Water	Table	Hate	Table	Water	Table	Water	Table	Kate	- Table
Parameter	Units	n	Max	n	Hax	n	Ках	n	Max	n	Max	n	Max	n	Max	n	Max	n	Kar	n	Max	n	Ман	n	Max
Antiwony Sb	<b>eg/</b> ]	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Arsenic As	ag/1	1	0.005	1	0.015	1	0.024	1	0.026	1	0.13	1	0.14	1	0.71	1	0.28	ı	0.08	1	0.27	!	0.08	1	0.055
Arsenic Filter Barium Ba	mg/l mg/l	-	-	-	-	-	-	-	-	_	-	-	-	_	-	-	-	-	_	-	-	_	-	-	-
Cadwium Cd	eg/l	1	(0.01	ı	0.03	1	0.02	1	0.05	1	0.18	1	0, 18	1	0.55	1	0.32	1	0.14	1	0.61	1	0.17	1	6.52
Cadeiue Filter	mg/l	-	•	•	-	-	-	-	-	-	-	-	-	-	•	-	-	-	-	-	-	-	-	-	•
Chloride Cl	mg/1	1	35	1	70	1	80	1	80	1	40	1	80	1	750	1	60	1	50	1	95	1	65	1	47
Chronium (Total:	•	-	•	-	-	-	-	-	-		-	-	-	-	-	-	-	-	-	-	•	-	-	-	•
Chrowium (Hex) Copper	mg/] mg/]	-	-	-	-	-	-	-	-	-	-	_	-	-	-	-	-	-	-	-	-	-	-	-	-
Cyanide	eg/l	-	_	_	-	-	-	-	•	_	-	_		-	_	•	-	_	-	-	-	-	-	-	-
Fluorade	mg/l	-	-	-	-	-	-	-	-	-	-	-	-	•	•	-	-	-	-	•	-	-	-	-	-
Iron Fe	mg/l	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	•	-	-	-	-	-	-
Lead Pb	mg/}	1	0.09	1	0.1	1	0.01	ı	0.24	1	0.09	1	0.01	1	2. 18	1	0.4	ŧ	2.93	1	0.31	ı	2.97	1	0.15
Lead Filter	mg/1	-	_	-	-	-	-	-	-	-	-	-		-	-	-	-	_	-	-	-	-	-	•	-
Manganese Mn Manganese Filter	mg/l r mg/l	_	_		-	-		_	-	-	-	-	-	-	-		-	-	_	• -	-	-	-	-	-
Mercury	ag/l	1	(0.0002	1	(0.0002	1	(0,0002	1	(0.0002	1	(0.0002	1	(0.0002	1	(0.0002	1	(0.0002	1	(0.0002	1	(0.0002	1	10.0002	1	(0.0002
Nickel	mg/1	1	0.07	1	0.18	1	(0.05	1	10.05	1	(0.05	1	(0.05	1	0.99	1	0.51	1	0.14	1	0.46	1	0.13	. 1	0.35
Selenium Se	mg/1	•	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Selenium Filter	<b>1</b> 99/1	-	-	-	•	-	-	-	• .	-	<b>-</b> .	-	-	-	-	-	•	-	•	-	-	-	-	-	-
Silver	<b>*9/1</b>	•	-	•	-	-	-	-	-	-	-	-	-	-	-	-	-	-	•	•	-	-	-	-	-
Sodium Sulfate 904	#g/l #q/l	-	400	-	1730	ī	1400	ī	2000	ī	1440		1950	ī	5080		6170	7	5690	•	1770	ī	3730	ī	2270
Sulfite	mg/1	:	-	:	-	:	-		-	-	-	:	-	:	-	:	-	-	-	:	-	-	-		-
Tin Sn	ng/l	•	-	-	-	-	-	-		-	-	-	-	-	-	-	•	-	-	-	-	-	-	-	•
Zinc	ag/l	1	0.35	1	0.41	1	0.3	1	0.36	1	0.98	1	1.1	1	4.5	1	2.1	1	2.2	1	4.0	1	2.3	ı	3.0
T.O.C.	mg/L	-	-	-	-	-	•	-	•	-	-	-	•	-	-	•	-	-	•	-	-	-	-	-	-
Surfactants R.O.D.	mg/) mg/1	-	-	-	_	-	-	-	-	-	-	-	-	-	-	-	-	-	•	-	-	-	-	-	-
C.O.D.	ag/1 ag/1	_	-	_		_		_	•	_	_	_	-	_	-	_	Ţ <b>\</b>	_	-	-	_	-	-	_	_
· Color		-	-	_	-	-	-	-	-	-	-	_	-	-	-	-	-	-	_	-	-	-	-	-	-
Hardness	mg/l (CaCol)	•	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	•	-	-	-	•
Odor	704	•	-	-	•	-	-	•	•	-	-	-	-	-	•	-	-	-	-	-	-	•	•	-	-
pH	8. U.	1	6.8	1	5.6	1	5.7	1	5.9	1	5.5	1	5.3	1	3.0	1	3.1	1	(1	ı	(1	1	1.7	1	3.0
Phenois T.D.S.	og/1 og/)	•	<b>502</b>	ī	20%	1	2641	ī	2654	•	1940	•	- 2517	1	6860	i	7490	1	5880	1	2440	•	5560	•	3160
Terbidity	MIN.	i	0.6	i	0.62	i	0.75	i	0.58	-	0.7	i	0.68	i	2.0	i	2.5	;	0.7	i	0.65	i	0.58	i	0.6
Fecal Coliform	Col/100 ml	-	-	-	-	-	-	_	-	-	-	-	-	-	-	-	-	_	-	-	•	-	-	-	•
Total Coliform	Coi/100 m}	•	•	-	-	-	•	-	-	-	-	-	-	-	-	-	-	-	•	-	-	-	-	-	-
Dil & Grease		-	-	-	-	-	-	-	•	•	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
NH3 -N	mg/1	-	-	-	-	-	3800	•	4000		- 2700	•	3250	-	7800	-	7800	:	-	-	3300	-	5800	-	4300
Conductivity TOH	microuchs ug/l	1	1000	1	<b>295</b> 5	1	3800	1	4U.U	1	2100 -	-	<b>3630</b>	1	7500	1	7800	1 -	7200		3300		2000	1	4300
Endrine	ug/1	-	-	_	_	_	-	_	-	_	_	_	_	_	-	-	-	_	-	_	-	_	_		_
Lindare	ug/1	-	-	-		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	_	-
Methoxychlor	wg/l	-	-	-	•	-	-	-	-	-	-	-	-	-	-	-	•	-	-	-	•	-	-	•	-
Toxaphrine	ug/1	-	-	-	-	-	-	•	•	•	-	•	-	-	-	-	-	-	-	-	•	-	-	-	-
2,4-8 2,4,5-TP	ug/l ug/l	-	-	-	-	•	-	-	-	-	_	-	-	-	-	-	-	•	•	-	-	-	-	-	-
C1417-16	चपु/ ।	P	-	-	-	•	-	•	-	•	-	•	-	•	-	•	-	•	-	•	-	-	7	-	-

### (continued) NSIU Pedricktown Ground Water Abatement System ANDIDANALYTICAL DATA

Table 8

		<b>EMU</b>	5 Mil	BAR	S NAZ	<b>GYM</b>	S MUS	DMF	S NE1	BHA	S NE2	EMA	S NE3	enuc	SW1	<b>EMU</b>	SW2	BHAS !	SE SPLIT	BHRS	SEI	GHAS	SES	enc	S SE3
		Wate	r Table	Water	r Table	Water	r Table	Wate	r Table	Wate	r Table	Wate	r Table	Water	Table	Water	r Table	Water	Table	Water	Table	Water	Table	Wate	r fable
Paraceter	Units		Ках	n	Max	ħ	Nan	n	Max	n	Kan	n	Нах	n	Max	h	Kax	n	Nax	n	Мак	n	Max	n	Наи
Gross Alpha	eCi/I	-	-	-	-	-	-	-	-	-	•	_	-	-	-	-	-	-	-	-	-	-	-	-	-
Grees Peta	pCi/l	-	-	-	-	-	-	-	-	-	-	-	-	•	-	-	-	-	-	-	-	-	-	•	-
Redium (total)	pCi/l	- ;	-	-	-	-	-	-	-	-	-	-	-	-	-	-		-	•	-	-	-	•	-	-
Radium 226	pCi/l	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	•	-	•
Ce-144	pCi/l	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ce-141	pCi/l	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	•.	-	-	-	•
₹e-103	pCi/l	-	-	-	-	-	•	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Cs-134	pCi/l	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ra-226 Bi	pCi/l	-	-	•	-	-	•	-	-	-	-	-	-	-	-	-	•	-	-	-	-	-	-	-	-
Ru-306	pCi/l	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	•	-	-
Cs-137	pCi/I		-	-	-	-	-	-	•	-	-	-	•	-	•	-	-	-	-	-	-	-	-	-	-
lr-95	pCi/l	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-		-	-	-	-
N5-95	pCi/1	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Co-58	pCi/l	-	-		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	•	-	-
Mn-54	pCi/l	-	`-	-	-	-	•	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Fe-59	pCi/l	-	-	-	-	-	-	-	•	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Zn-65	pCi/I	-	-	-	-	-	-	-	-	-	-	-	•	-	-	-	-	-	-	-	-	-	•	-	-
Co-60	pCi/l		-	-	-	-	-	-	•	-	-	-	-	-	-	-	-	-	-	-	•	•	-	-	-
K-40	pCi/l	_	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Cr-51	pCi/i	-	•	-	-	-	-	-	-	-	· <b>-</b>	-	-	-	-	-	-	-	•	-	-	-	•	•	-
I-131	pCi/I	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ba-140 La	pCi/S	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

图 17 中国 国 图 图 图 图 图 图 图 图 图 图 图 图 图 图 图 图

<sup>-</sup> Samples collected by Mt. Industries, Inc.; analysis conducted by Century Environmental Labs, Inc., Therefore, NJ.

Table 9 MSNJ Pedricktown Residential Hells Ground Water Quality Data

										2.0											
		Hell	PAIL	We 1	PH2	He l	1 PN3	Wel	l PHA	He11	P./5	Well	PM6	Well	PV7	We31	PMB	Well	PMS	Marsha	11 Well
		Hater	Table	Water	r Table	Wate	r Table	Wate	r Table	Water	Table	Water	Table	Water	Table	Water	Table	Water	Table	Water	Table
Panareter	Units	n	Nax	n	Ман	n	Max	n	Max	ħ	Max	n	Мах	n	Max	n	Max	n	Max	n	Max
Antiscopy 55	wg/i	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1	0.60	-	•
Arsenic As	mg/1	3	(0.005	-	0.005	3	(0.005	3	0.005	3	(0.005	5	10.005	2	(0.005	-	-	3	0.006	-	-
Arsenic Filter	mg/1	-	-	-	-	3	0.11	3	0.14	3	0.09	5	0.15	2	0.18	•	-	3	0.135	3	(0.005
Rarium Ra Cadmium Cd	eg/1		0.0l 0.0≥	3	0. 15 0. 01	3	(0.001	3	0.01	3	0.03 0.01		0.01	Ş	0.01	•	-	3	0.02	ĭ	0.01
Cadmium Filter	mq/  mg/	-	-	-	-	-	-	-	-	_	-	-	-	-	-	-		-	-	-	-
Chloride Cl	mg/1	-	-	-	•	-	-	-	-	-	-	-	-	-	-	-	-	-	•	-	-
Diremium (Total)	mp/l	3	0.06	3	0.01	3	10.01	3	0.015	3	0.32	2	0.015	5	0.015	•	•	3	0.015	4	0.05
Chromium (Hex)	mq/1	-	•	-	-	-	-	-	-	-	-	-	-	3	-	-	_	3	0.33	3	0.30
Copper	mg/1		0.34	3	0.29	3	0.18	. 4	0.50	3	0.39	5	0.25		0.36		-		v. 33		-
Cyanide Fluoride	wg/1	_	-	_	_	-	-	-	_	_	_	-	-	•	-	_	-	•	-	-	-
iron Fe	mg/l mg/l		12.3	3	13.5	3	5.22	3	35.0	3	0.48	2	40, 4	2	1.0	-	•	3	0.5	4	0.264
Lead Pb	<b>19/1</b>		0.9	4	0.40.	3	10.01	3	(0.01	3	0.028	8	10.01	2	0.01	-	-	3	10.01	4	0.11
Lead Filter	mg/1	-	-	•	•	-	-	-	•	-	-	-	-	-	-	•	-	:	-	-	-
Hanganese Mn	mg/	3	0.3	3	0. 15	3	0.118	3	0.3	3	0,04	Ş	7.95 -	5	0.08	-	_	3	1.46	•	0.055
Manganese Filter		_	-	-		-		•	0.010	4	0.011	3	0.012	Ł	0.012	-		3	0.01	3	10,0002
Hercury Nitrate	mg/1 mg/1	-	0.15	•	0.015	4	0.011	•	0.012	-		-	-	-	-	-	-	3	-		-
Selenium Se	ag/1	Ş	0.007	3	0.026	3	0.018	5	0.003	3	(0.003	2	ND -	5	(0.003	-	-	•	0, 109	3	0.006
Selenium Filter Silver	mg/l mg/l	3	10.005	3	(0.005	3	(0.005	1	(0.005	3	(0.005	5	(0, 005	2	(0, 005	1	0.20	3	(0, 005	4	0.01
Sodium	eg/)	-	13.3	j	45.8	3	6.4	ż	3.1	-	2.9		71.4	٤	3.7	-	•	3	35.5	4	11.6
Sulfate 904	mg/)		-	-	-	-	_	-	-	-	-	-	•	-	-	-	-	-	-	-	-
Sulfite	<b>ug/1</b>	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	•	-	-
Tin Sn	mg/1		-	-	-	- •	-	-	-	-	-	3	-	3	-	-	0.97	ī	2.6	ī	0.22
Zinc	mg/1		1.0	•	1.8	4	0.65	4	0.98		0.89	-	1.2	-	1.0		-	•	-	-	V-EE
T.O.C. Serfactants	mg/l mg/1 LAS	-	-	-	_	-	_	_	_	_	_	_	-	_	-	<b>-</b> ,	-	•	-	_	-
9. C. D.	mg/1 C/C	_	-	-	_	-	-	_		-	•	-	-	-	-	-	-	-	-	-	-
C. O. D.	mg/l	-	-	•	-	-	-	•	-	-	-	-	-	-	-	-	-	-	•	-	-
Color		-	-	-	-	-	-	-	-	-	-	•	•	-	-	-		•	-	-	-
Hardress	mg/1 (as Ca003)		-	-	-	-	-	-	•	_	-	-	-	-	-	-	-	-	-	-	
Odor pH	70N S.U.	-	-	-	-	-	-	-	<b>-</b> ,	-	_	-	-	_	_	-	_	-	_	-	•
Phenols	<b>69/1</b>	_	-	_	-	_	-		_	•	-	-	-	-	<b>-</b> ,	•	-	-	-	-	-
T. D. S.	mg/1	-	-		-	-	-	-	-	-	-	-	-	-	-	-	• .	-	-	-	-
Terbidity	NTU	-	<b>-</b> .	-	-	-	-	-	-	-	-	-	- '	•	-	-	-	•	-	-	-
Fecal Coliforn	Co1/100 ml	-	-	-	•	-	•	-	-	-	• .	-	-	-	-	•	-	-	-	-	-
Total Coliforn	Co1/100 ml	-	-	-	-	-	-	-	_	-	-	-	-	-	:	-	-		-	-	-
Dil & Grease Mi3 -N	mg/l	-	-	-	-	_	_	-	•	_	-	-	-	_	_	_		_	_	-	-
Corductivity	micromops	-	_	-	-	-	-	٠.	-	-	-	-	-	•	-	•	-	-	-	-	-
TOH	wg/1	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	•	-	-	•
Endrin	ug/1	•	-	-	-	-	-	-	-	•	-	. •	-	-	•	-	-	-	•	~	-
Lindane	eg/1	-	-	•	•	-	-	-	-	-	-	•	-	•	-	-	-	-	-	-	-
Methoxychlor	ug/1	-	-	•	_	-	-	-	_	-	-	:	-	-	-	•	_	_	-	-	
Toxaphrine 2,4-B	ug/) ug/)	-	-	-	_	-	-	-	-	-	-	•	-		•	-	-	-	•	-	-
2,4,5-TP	ug/1	-	•	-	-	-	-	-	-	•	-		-	-	-	-	•	-	- '	-	•
-, ', - ''	-1											•				•					

Table 9 (continued) NSNJ Pedricktown Residential Wells RADIDANALYTICAL DATA

•		Hell	PHI	He l	1 PNS	Well	PH3	Hel	1 PW4	Well!	PM5	Hel I	PN6	Hell	PW7	Hell	PHB	Well	PW3	Narsh	all Well
		Water	Table	Wate	r Table	Water	Table	Water	r Table	Hater	Table	Water	Table	Nater	Table	Water	Table	Water	Table	Hate	r Table
Paraveter	Units	· n	Max	7	Max	n	Rax	n	Hax	n	Kan	n	Max	h	Max	n	Max	n	Ман	n	Max
Gross Alpha	pCi/l	5	1.76 0.62	2	2.87 0.86	5	1.95 0.41	2	2.28 0.62	2	0.68 0.39	2	3.91 2.2	£	0.81 0.30	1	1.34 0.39	2	3.91 2.08	-	-
Gross Beta	pCi/l	2	1.99 0.36	5	3.48 0.42	5	2.31 0.23	5	2.11 0.29	2	2.81 0.25	5	8.17 1.91	2	3.41 0.31	1	0.52 0.86	2	33.7 2.5	~	-
Radium (tetal)	pCi/l	• 1	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Radium 226	pCi/l	1	0.40 0.17	1	0.02 0.12	1	0.47 0.1	8 1	0.26 0.1	5 1	0.10	1	1.27 0.25	1	0.14 0.1	4 -	-	1	1.33 0.25	-	-
Ce-144	pCi/I	1	(= 38.5	1	(= 36.7	1	(= 37.7	1	(= 38.6	1	(= 38, 6	1	(= 35.4	1	(= <b>36.</b> 7	1	(= 22.3	1	(= 40.8	-	-
Ce-141	pCi/l	1	(= 16.9	1	(= 14.1	ı	(= 14. f	1	(= 17.4	1	(= 17.7	1	(* 16.5	1	(= 47.4	1	(= 14.8	1	(= 28.0	-	-
1-131	pCi/l	1	(= 96.0	t	(= 56.6	1	(= 51.4	1	(= 100	1	(= 110	1	(= 112	1	MOA N/A	t	(= 404	1	(= 300	-	-
Ru-103	pCi/i	1	(= 9.80	1	(= 9.80	1	(= 8.74	1	(= 10.2	1	(= 9.95	1	(= 9,44	1	(= 23.7	1	(* 8.72	1	(= 16.2	-	-
Cs-134	pCi/l	1	(= <b>5.9</b> 9	1	(= 6.99	1	(= 4.97	1	(= 7.40	1	(= 6.95	1	(= 6.49	1 .	(= 4.44	1	(= 2.86	1	(= 5.29	-	-
Ra-266 Di	pCi/l	1	(* 18. 1	1	(= 18.1	1	(= 9.92	1	(= 17.9	t	(= 17.B	1	(= 16.8	1	17 12	1	(= 5.49	1	(= 10.7	-	•
Ru-106	pCi/l	1	(= 58.0	1	(= 58.0	1	(* 49.2	1	(= <b>5</b> 3.6	1	(= 56.3	1	(* 50.2	1	<b>(= 46.3</b>	1	(= 28.7	1	(= <b>51.5</b>	-	-
Cs-137	pCi/l	1	(= 6.20	1	(= 6,20	1	(* 5.68	1	(= 6.38	1	(* 6.24	1	(= 5.74	1	(= 4.67	1	(= 3.07	•	(= 5.55	-	-
2r-95	pCi/l	ı	(= 13.6	1	(= 13.6	1	(= 11.3	1	(= 14.5	1	(= 13.8	1	(= 13.3	1	(= 20.3	t	(= 8.%	1	(= 17.E	-	-
Nb-95	pCi/l	1	I- 8.43	1	(= 8.43	1	(= 7.13	ŧ	(= 8.52	1	(= 8.89	1	(= 7.81	1	(= 11.6	i	(= 5.11	ŧ	(* 9.92	-	-
Co-58	pCi/l	1	(= 7.86	ı	(= 7.86	1	(= 5.99	1	(= 8.81	t	(= B.22	ı	(* 7.65	t	(= 9.80	ŧ	(= 4.61	1	(* 8.50	•	-
Mn-54	pCi/l	1	(= 6.77	1	(= 6.77	1	(= 5.11	1	(= 6.63	1	(= 6.63	1	(* 5.86	1	(= 5.09	1	(= 3.03	1	(* 5.81	•	•
Fe-59	pCi/l	1	(= 18.8	1	(= 18.8	1	(- 14.2	1	(= 19.8	1	(= 18.7	1	(= 17.4	1	(= 33.6	1	(= 12.6	1	(= 24.2	_	·
2n-65	pCi/l	1	(= 16.3	1	(= 16.3	1	(= 11.6	1	(= 17.5	1	(= 17.0	1	(* 15.3	- 1	(= 11.1	i	(= 6.62	1	(= 13.2	-	-
Co-60	pCi/l	1	(= 5.97	1	(= 5.97	1	(= 5.22	1	(= 6.03	1	(= 5.4)	1	(= 6.04	1	(= 4.54	i	(= 2.99	1	(= 5.55	_	-
N-40	pCi/1	1	(= 14B	1	390 30	1	340 90	1	(= 147.	1	(= 154	1	(= 142	1	(* 52.0	ŧ	(* 26.5	1	(= 16.9	-	-
Cr-51	pCi/I	-	•	_	•	-	-	-	-	-	-	•	-	-	•	-	•	-	•	-	_
1-131	pCi/l	-	•	_	-	_	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ba-140 La	pCi/i	-	•	-	-		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

<sup>-</sup> Samples collected and analyzed by New Jersey Department of Environmental Protection.

Well No.	Land Surface Elevation	Top of Casing Elevation	Screened Interval Elevation Below Land Surface	Length of Screened Interval	Lithology of Screened Interval	Construction Date of Well	Construction Hethods for Wells Listed
HD	14.13	16.73	-9.67 to -24.67	151	sand; silt	December, 1982	Drilling Method: auger
HS	14.23	16.83	4.83 to -10.17	151	silt; sand	December, 1982	
ID	12.6 <sup>F</sup>	15.24	-5.96 to -20.96	15 '	sand	December, 1982	Sampling Method: Split-spoon
15	12.91	15.41	7.41 to -0.09	101	sand; silt	December, 1982	sampling at 5° intervals in wells
JD	9.18	12.08	-5.92 to -15.92	10'	sand	December, 1982	completed in the lower water table
JS	9.35	11.95	4.95 to -5.05	101	sand	December, 1982	zone.
KD	8.1	10.70	-7.3 to -17.3	10'	sand	December, 1982	
KS	8.01	10.51	2.51 to -7.49	10'	sand	December, 1982	Casing: 2" ID; Sch. 40 PVC with
LD	8.59	10.89	-1.11 to -8.11	7'	sand; clay	December, 1982	20 slot screan
LS	8.64	10.74	4.74 to -2.31	7'	sand	December, 1982	
MD	6.37	8.37	-3.23 to -11.23	8,	silt; sand	December, 1982	
MS	7.03	9.83	3.83 to -3.17	7'	sand; silt	December, 1982	
ND	8.25	10.35	-3.65 to -13.65	10'	sand	December, 1982	
NS	8.7	11.30	4.5 to -5.5	101	sand	December, 1982	
<b>O</b> D	8.44	11.44	-11.06 to -26.06	15'	sand	December, 1982	
PD	7.05	10.92	-9.75 to -19.75	101	sand	December, 1982	
P\$	7.04	10.25	-0.86 to -10.86	10'	sand	December, 1982	
<b>Q</b> D	7.69	9.14	-3.81 to -13.81	10'	sand	December, 1982	
QS	7.92	10.19	5.52 to -4.48	101	sand	December, 1982	
RD	11.62	13.62	-13.38 to -23.38	10'	clay; silt	December, 1982	
RS	11.84	13.84	6.84 to -8.16	15'	sand; clay	December, 1982	
SD	8.95	11.45	-6.05 to -18.05	12'	sand	December, 1982	
SS	8.76	10.76	3.76 to -6.24	10'	sand	December, 1982	
T2	8.94	11.34	1.34 to -13.66	151	sand	December, 1982	
<b>★</b> T4	9.09	11.09	1.09 to -13.91	151	sand	December, 1982	NOTE: *T4 = Casing: 4" ID; Sch.

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NOTE: \*T4 = Casing: 4" ID; Sch.

80 PVC with 20 slot screen

### OBSERVATION and MONITORING WELL DATA SUMMARY

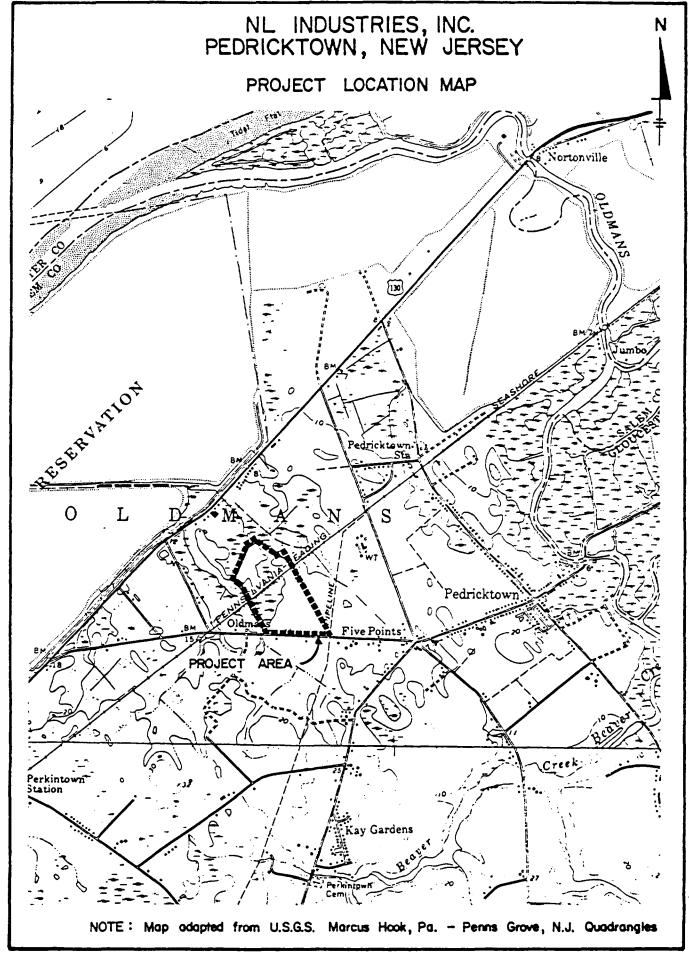
Well No.	Land Surface Elevation	Top of Casing Elevation	Screened Interval Elevation Below Land Surface	Length of Screened Interval	Lithology of Screened Interval	Construction Date of Well	Construction Methods for Wells Listed
1R	9.32	13.32	5.32 to -22.68	28'	sand; silt; sand; clay	December 14, 1979	Mud rotary - Ditch & split-spoon sampling; 4" ID; PVC, 20 slot screen
2R2	6.94	9.14	-6.06 to 13.06	7'	sand; gravel; clay	October 15, 1980	Water rotary - Ditch sampling; 4" ID; PVC; 20 slot screen
3R	11.4	14.10	7.4 to -21.6	29"	sand; clay; sand; clay	December 17, 1979	Mud rotary - Ditch sampling, 4"
							ID; PVC; 20 slot screen
4R	12.1	14.80	3.1 to -8.9	12'	sand; clay	July 21, 1980	Water rotary - Ditch sampling; 4" ID; PVC; 20 slot screen
5R	8.03	10.03	1.03 to -7.7	91	sand; clay	July 22, 1980	Water rotary - Ditch sampling; 4" ID; PVC; 20 slot screen
6	9.73	12.23	-1.27 to -11.27	10'	sand; clay	October 8, 1976	Auger; split spoon sampling; 4" ID; PVC; 40 slot screen
Sec BR	13.65	16.55	-87.35 to -94.35	7'	sand; clay	March 19, 1980	Mud Rotary; Split spoon sampling; 4" ID; PVC; 20 slot screen
9R2	13.93	16.73	-39.07 to -47.07	81	sand; clay	April 2, 1980	Water Rotary; Unknown sampling method; 4" ID; PVC; 20 slot screen
AR	8.89	11.39	6.39 to -23.61	30'	sand; clay	December 12-13, 1979	Mud Rotary; Ditch and Split spoon sampling; 4" ID; PVC; 20 slot screen
BR	6.58	8.88	-24.42 to -30.42	6'	sandy clay; sand	April 1, 1980	Water Rotary; Ditch sampling; 4" ID; PVC; 20 slot screen
CR2	13.16	15.96	-11.84 to -17.84	6'	sand	March 29, 1980	Water Rotary; Ditch sampling; 4" ID; PVC; 20 slot screen
6 th 10	11.72	13.72	-30.28 to -60.28	30,	sand; clay; sand	*	Auger; split spoon and Shelby tube sampling; 4" ID; PVC; 20 slot screen
K-) \ 11	7.45	9.25	-25.75 to -45.75	201	sand	*	Auger; split spoon sampling; 4" ID; PVC; 20 slot screen
→ 11R	* *	*	*	20'	sand; clay	October 14, 1983	Drilling method unknown; sampling method unknown; 4" ID; 20 slot screen

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## **Figures**

NLI 001 0175

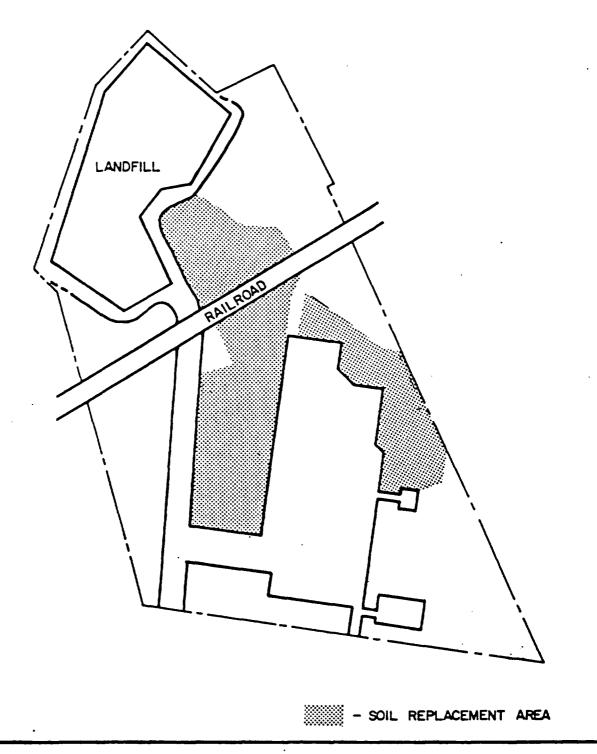


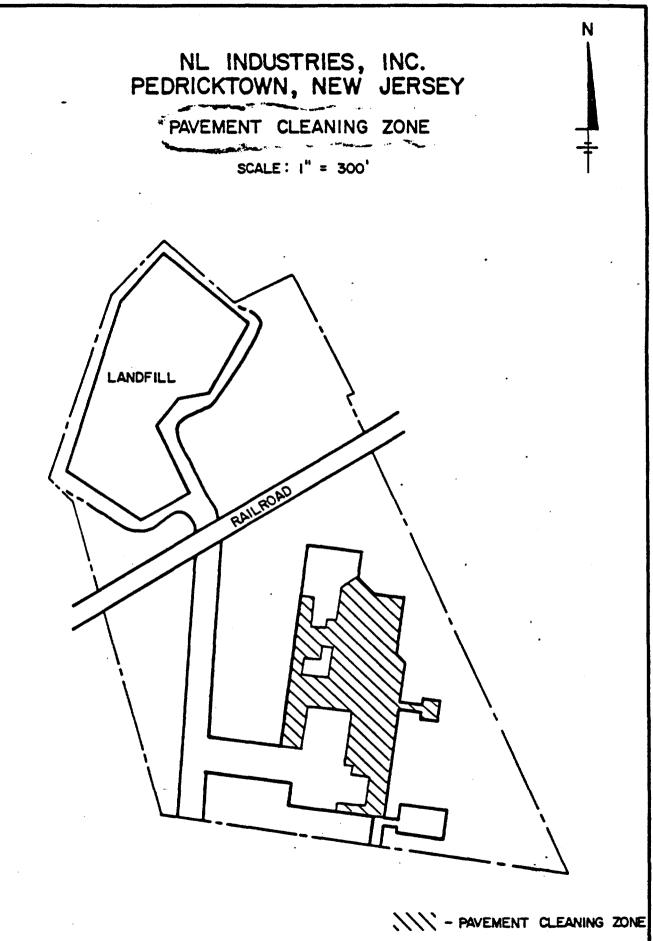
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## NL INDUSTRIES, INC. PEDRICKTOWN, NEW JERSEY

SOIL REPLACEMENT AREA

SCALE: I" = 300'



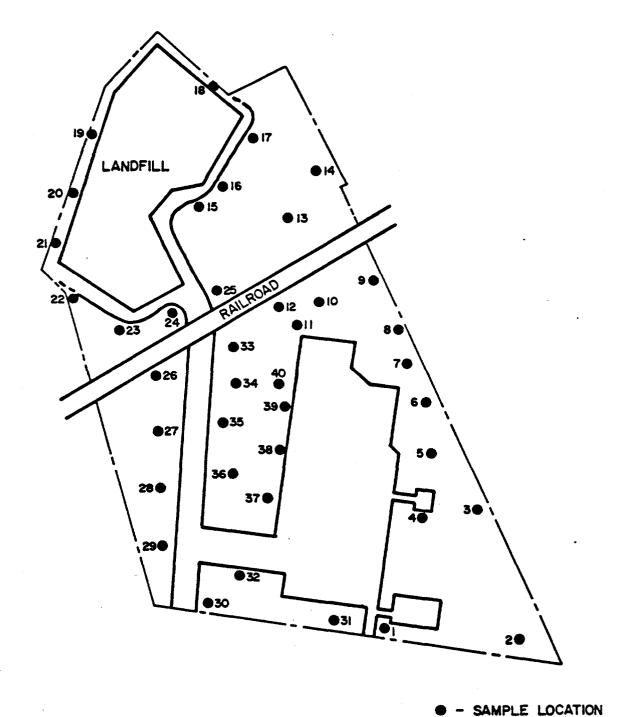


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## NL INDUSTRIES, INC. PEDRICKTOWN, NEW JERSEY

SOIL SAMPLING LOCATIONS (1980)

SCALE: I" = 300'



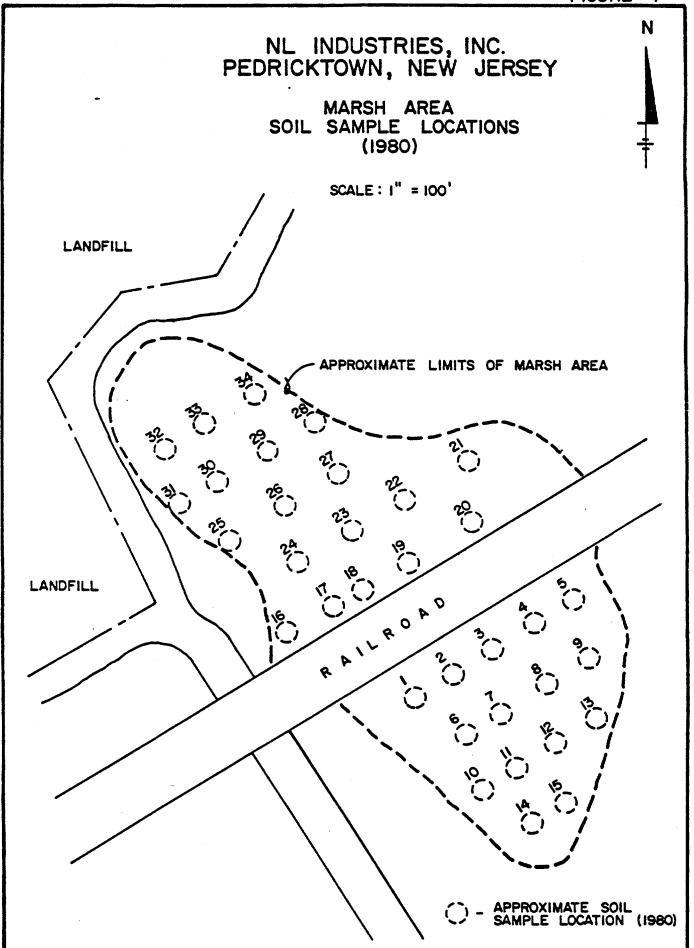
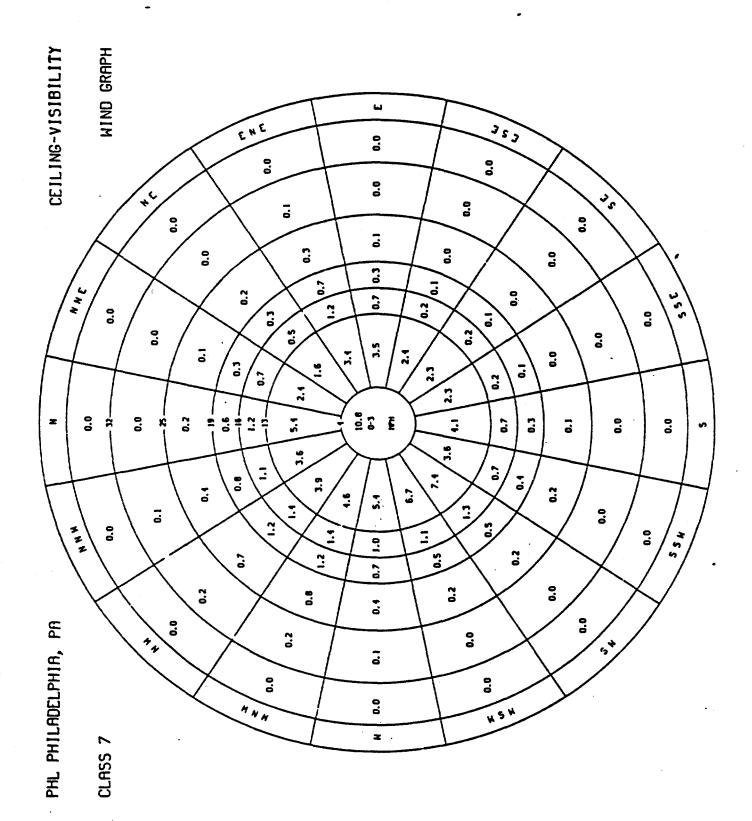


FIGURE 9 - WIND ROSE FOR PHILADELPHIA, PA



# **Exhibits**

NLI 001 0182

# EXHIBIT A LANDFILL CLOSURE INFORMATION

J.

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#### NSNJ PEDRICKTOWN SITE

#### CHRONOLOGY OF EVENTS RELATIVE TO ON-SITE LANDFILL

Plant Soils and Marsh Soils Cleanup Activities Started.

December 1982

Started Placement of Landfill Clay Cap to Reduce Leachate Generation.

August 1983

Clay Cap Completed and Tested August 10, 1983.

October 20, 1983

Closure Plan Approved by NJDEP.

December 15, 1983

Closure Certified by NL Industries, Inc. and Roy F. Weston, Inc.

December 22, 1983

Landfill Maintenance Operations Transferred to NSNJ.

May 29, 1984

NL Repairs Winter Damage to Landfill Cover.

June 15, 1984

National Bank of Georgia Terminates Final Employee.

NL Voluntarily Enters Site to Maintain Landfill.

January 25, 1985 Phase B - Automatic Operation Commences.

February 20, 1985 Phase A - Automatic Operation Commences.

6:9

August 1982

June 18, 1984



## State of Rem Jerney

#### DEPARTMENT OF ENVIRONMENTAL PROTECTION

DIVISION OF WASTE MANAGEMENT 32 E. Hanover St., CN 027, Trenton, N.J. 08625

DR. MARWAN M. SADAT

October 20, 1983

LINO F PEREIRA

Mr. Stephen W. Holt N. L. Industries, Inc. Penns Grove - Pedricktown Road Pedricktown, New Jersey 08067

RE: N. L. Industries, Inc., Oldmans Township, Salem County, New Jersey Facility Registration Number 1706C

Dear Mr. Holt:

The Bureau of Hazardous Waste Engineering has reviewed the submittals from Roy F. Weston, Inc. made on December 15, 1982, March 24, 1983 and August 16, 1983 in behalf of N. L. Industries, Inc. in reference to the Closure and Post-Closure plans for a hazardous waste landfill located at the former N. L. Industries, Inc. Pedricktown plant premises. This submittal was made in response to items 2(d) and 3 of the Administrative Consent Order signed by N. L. Industries, Inc. and the Department on October 6, 1982 regarding the final closure of the existing hazardous waste landfill.

The Bureau of Hazardous Waste Engineering has found said submittals in conformance with regulatory standards set forth in Subchapters 9 and 11 of N.J.A.C. 7:26 and hereby approves the closure and post-closure plans subject to compliance with the following conditions:

- 1) The closure shall follow specifications of the engineering designs and narrative prepared by Roy F. Weston, Incorporated; specifically: the engineering report prepared by James H. Dougherty, P.E. and Amir A. Metry, P.E. dated December 13, 1982, the engineering report in response to NJDEP comments prepared by Michael H. Corbin, P.E., dated March 22, 1983, drawing sheet No. 5 Rev. No. 5 prepared by Roy F. Weston, Inc. dated June 24, 1977, drawing sheet No. 6 Rev. No. 3 prepared by Roy F. Weston, Inc. dated September 11, 1981, drawing sheet No. 7 Rev. No. 3 prepared by Roy F. Weston, Inc. dated September 11, 1981 and drawing sheet No. 8 Rev. No. 1 prepared by Roy F. Weston, Inc. dated September 11, 1981, except as modified herein.
- 2) The landfill shall be graded in accordance with the narratives prepared by Roy F. Weston, Inc. dated December 13, 1982 and as shown on drawing sheet No. 5 Rev. No. 5 dated June 24, 1977, prepared by Roy F. Weston, Inc.

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- 3) After grading, a clay cap of a minimum thickness of six (6) inches and maximum permeability of 1 x 10 cm/sec shall be placed over the entire landfill surface and compacted to 90% minimum as per ASIM / D-1557 Method C. This item is expected to be completed before October 30, 1983 and must be certified by a New Jersey Licensed Professional Engineer upon implementation.
  - 4) A final cover system, designed to intercept and divert any infiltration away from the clay cap layer, shall be placed on top of the clay cap. The bottom layer of this cover system shall consist of a 6-inch layer of a coarse to fine sand with a permeability of 1 x 10<sup>-2</sup> to 1 x 10<sup>-3</sup> cm/sec. Six inches of clean earth shall be placed over this bottom layer followed by six inches of top soil. These soil layers shall serve to support a vegetative cover.

The final elevation of the landfill shall be approximately 40 feet above MSL (mean sea level). The final landfill contours and drainage plan shall be as shown on drawing sheet No. 6 - Rev. No. 3 prepared by Roy F. Weston, Inc., dated September 11, 1981. The final side slopes shall be as described in the narratives prepared by Michael H. Corbin, P.E. (design clarification report) dated August 11, 1983. The side slopes on the western and southern boundaries of the landfill shall not be steeper than 2:1 (horizontal to vertical), ranging from approximately 4:1 to 2 1/2:1. Other slopes on the eastern and northern boundaries shall be 2:1 or less steep. This item is expected to be completed by October 30, 1983 and must be certified by a New Jersey Licensed Professional Engineer upon implementation.

5) The top of the landfill shall be graded at a minimum slope of 1 percent to provide positive drainage.

Surface run-off in Phase A shall be directed to a corrugated metal half-section pipe drain down the side slope as shown on drawing sheet No. 6 - Rev. No. 3 prepared by Roy F. Weston, Inc. and dated September II, 1981. Run-off shall then be conveyed by storm sewer or stone-lined channels to existing waterways. Surface run-off from Phase B shall be directed to a stone-lined swale down the side slope as shown on said drawing sheet, and then across the perimeter road to surface discharge. Diversions shall be constructed around the leachate sumps' riser pipes pumping pads. This item is expected to be completed by October 30, 1983 and must be certified by a New Jersey Licensed Professional Engineer upon implementation.

6) A drainage swale shall be constructed along the landfill perimeter to collect run-off from the side slopes. This storm-water shall be dissipated across the perimeter road by stone swales at the natural low-points. Design details for this drainage system shall be as shown on drawing sheet No. 8 - Rev. No. 1 prepared by Roy F. Weston, Inc. dated September 11, 1981. Completion of this item is expected by October 30, 1983 and must be certified by a New Jersey Licensed Professional Engineer upon implementation.

fully sking

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- 7) The top soil shall be stabilized to prevent erosion by establishing a vegetative cover. This vegetative cover shall be established by spreading a type A seed mixture, and shall be mulched with straw, in accordance with the New Jersey Department of Transportation (N.J.D.O.T.) specifications. The side slopes shall be seeded with N.J.D.O.T. type E seed mixture, which includes crown vetch, and shall be mulched to minimize the potential of soil erosion until vegetation is established. Completion of this item is expected by October 30, 1983 and must be certified by a New Jersey Licensed Professional Engineer upon implementation.
- 8) A permanent fence shall be installed around the landfill and all monitoring wells. The fence shall be at least 8-feet high and topped with three strands of barbed wire. Vehicle access gates with locks shall be installed at the landfill entrance read and directly north of the leachate storage tank. Completion of this item is expected by October 30, 1983 and must be certified by a New Jersey Licensed Professional Engineer upon completion.
- Phase A and Phase B shall be removed, to restore these sumps to effective monitoring points for assessment of the integrity of the primary (upper) landfill liner. Completion of this item should be undertaken by October 30, 1983 and must be certified by a New Jersey Licensed Professional Engineer upon completion.
- 10) The owner of the property on which a disposal facility is located shall record, in accordance with State Law, a notation on the deed to the facility property, or some other instrument which is normally examined during title search, that will in perpetuity notify any potential purchaser of the property that:
  - a) The land has been used to manage hazardous waste.
  - b) Post-closure use of property on or in which hazardous waste remains after closure shall never be allowed to disturb the integrity of the final cover, liners, or any other components of any containment system, or the function of the facility's monitoring systems, unless the owner or operator can demonstrate to the Department by petition that the disturbance:
    - i) Is necessary to the proposed use of the property, and will not increase the potential hazard to human health or the environment; or
    - ii) Is necessary to reduce a threat to human health or the environment.
  - c) The survey plat and record of the type, location, and quantity of hazardous waste disposed of within each cell or area of the facility required in paragraph 12 of this approval have been filed with the local zoning authority or the authority with jurisdiction over local land use and the Department.

- II) If at any time the owner or operator or any subsequent owner of land upon which a hazardous waste facility is located removes the waste and waste residues, the liner, if any, and all contaminated underlying and surrounding soil, the owner or operator may add a notation to the deed or instrument indicating the removal of the waste.
  - 12) Within 90 days after closure is completed, the owner or operator of a disposal facility shall submit to the local land authority and to the Department a survey plat indicating the location and dimension of landfill cells or other disposal areas with respect to permanently surveyed benchmarks. This plat shall be prepared and certified by a professional land surveyor. The plat filed with the local land authority shall contain a note, prominently displayed which states the owner's or operator's obligation to restrict disturbance of the site as specified in paragraph 10b of this approval. In addition, the owner or operator shall submit to the Department and to the local land authority a record of the type, location, and quantity of hazardous wastes disposed of within each cell or area of the facility.
  - 13) Within 90 days after final closure, NL Industries shall submit to the Bureau of Hazardous Waste Engineering a final topographic map of the landfill. This item must be prepared by a New Jersey Licensed Land Surveyor.
  - licensed Professional Engineer's certifications and other submittals required by this approval have been submitted to the Department and approved, and an acceptable post-closure care plan has been approved by the Department. In the interim, NL Industries, Inc., shall continue to comply with all monitoring and reporting requirements of N.J.A.C. 7:26-1 et seq. and the Engineering Design Approval of record for the subject landfill.
- NL Industries, Inc. shall establish within thirty (30) days from the date of this approval, compliance with the liability requirements of N.J.A.C. 7:26-9.13, including the submission to the Department of originally signed duplicates of the insurance policies for sudden and accidental occurrences required by N.J.A.C 7:26-9.13(b) and for non-sudden and accidental occurrences required by N.J.A.C 7:26-9.13(c).
- The New Jersey Licensed Professional Engineer's certifications required by this approval shall be submitted to the Department within seven all days after the item being certified has been completed, or within seven all days of the date of this approval, if the item has already been completed.

These certifications, along with all submittals to the Department to be made pursuant to conditions 12, 13, 14, and 15 of this approval shall be addressed to:

Frank Coolick, Chief
Bureau of Hazardous Waste Engineering
Division of Waste Management
New Jersey Department of Environmental Protection
CNO28
Trenton, New Jersey 08625

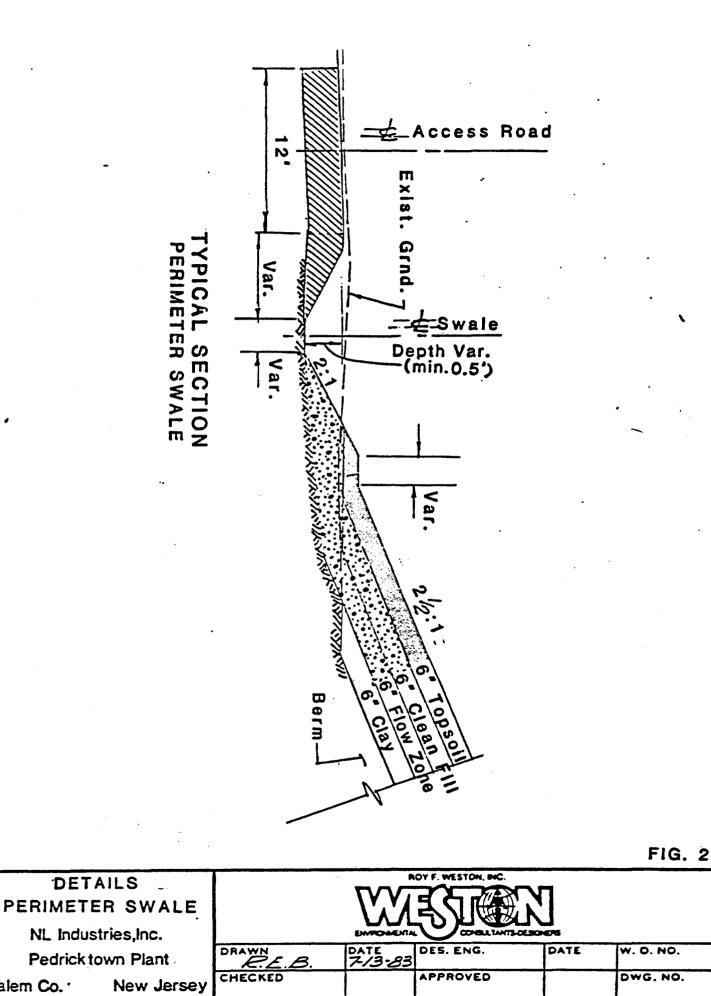
If you have any questions regarding these matters, please call the Bureau of Hazardous Waste Engineering at (609) 292-9880.

Very truly yours,

Edward Jy Londres Assistant Director

Engineering

EP8/EP10/jb



Salem Co.



**WESTON WAY** WEST CHESTER, PA. 19380 PHONE (215) 692-3030 TELEX: 83-5348

13 December 1983

Mr. Stephen W. Holt N.L. Industries, Inc. Penns Grove-Pedricktown Road Pedricktwon, NJ 08067

W.O.#0793-08-04

Reference: Landfill Closure Certification ·

N.L. Industries, Inc. Pedricktown, New Jersey Facility No. 1706C EPA I.D. #NJD061843249

Dear Steve:

This is to certify that the closure of the landfill referenced above has been completed by Haas Construction Inc. (prime contractor) in accordance with the following closure plan documents:

- Closure/Post-Closure Plan dated 12/13/82, submitted 12/15/82, prepared by WESTON.
- Addendum report to Closure/Post-Closure Plan dated 3/22/83, submitted 3/24/83, prepared by WESTON in response to NJDEP comments on 12/13/82 Closure/Post-Closure Plan.
- 3. Design clarification letter report dated 8/11/83, submitted 8/16/83, prepared by WESTON. and
- 4. Engineering Design Approval (NJDEP document) dated 4/5/78.

In accordance with the October 27, 1983 meeting between N.L. Industries, WESTON and NJDEP, WESTON provides the following certification for the landfill closure work:

A clay cap of a minimum thickness of 6 inches with clay having a maximum permeability of lx10-7 centimeters per second has been placed over the entire landfill surface and has been compacted to at least 90% as determined by the ASTM D698 method. The attached (Attachment #1) certified laboratory results



Mr. Stephen W. Holt N.L. Industries, Inc.

-2-

13 December 1983

provided by the Contractor (U.F.&S Reference No. 4617) provides the results of the permeability testing of nine clay samples and indicates thickness of the clay cap at these locations.

- 2. A final cover system consisting of a gravelly sand and sand flow/fill zone and a 6 inch top soil layer was installed and the flow zone meets or exceeds the permeability standards set forth in the closure plan documents. The certified analysis (provided by the Contractor, U.F. &S Reference No. 4580) of the cover system materials is included as Attachment #2.
- 3. The final elevation of the landfill does not exceed 45 feet above mean sea level.
- 4. The side slopes of the landfill are within the limits established by the closure plan documents.
- 5. The final gradient of the completed final cover on the top area of the landfill was measured to be a minimum of 1%.
- 6. Drainage structures including retaining walls around the leachate collection sumps and slope drains were installed and completed in accordance with the closure plan documents.
- 7. Seeding of the final cover was performed and completed in accordance with the specifications. The application rates were established by the Contractor and provided in Attachment \$3 (U.F.&S Reference No. 4669-A). The slopes were planted with 100 lbs. of annual rye and 25 lbs. of crown vetch per acre. The top was planted with 50 lbs. of perennial rye, 30 lbs. of Kentucky 31 and 30 lbs. of chewing fescue per acre. Vegetative cover was established exclusive of some areas which may require reseeding in the spring under the post closure plan.
- 8. An 8 foot high chain link security fence has been installed surrounding the landfill.
- 9. Attachment #4, Memorandum from R. Buss to M. Corbin dated November 18, 1983 details the procedure for sounding of the liquid levels in the monitoring sumps in accordance with our meeting of October 27, 1983 and

WESTEN

Mr. Stephen W. Holt N.L. Industries, Inc. -3- 13 December 1983

provides the initial certified sounding measurements from the top of the riser pipes to the liquid levels as recorded on November 18, 1983. A sounding measurement of 28.3 feet was recorded in Sump A and a sounding measurement of 29.9 feet recorded in Sump B.

If WESTON can provide additional information on the closure work performed at the Pedricktown Plant Landfill please contact the undersigned.

Very truly yours,

ROY F. WESTON, INC.

Michael H. Corbin, P.E.

Project Manager

James H. Dougherty, P.E.

Vice President

MHC/JHD/snh

Attachments

cc: Mr. Dean Ervin
 Jeffrey Jacobs, Esq.

3 South Black Horse Pike Mt. Ephraim, N. J. 08059

William R. Underwood, P. E.

William M. Furman, Manager

Soil Borings - Soil Engineering - Testing - Inspection - Concrete - Steel - Asphalt - Masonry

CLIENT: Haas Construction Company

PROJECT: N.L. Industries

TEST/INSPECTION REQUIRED: Clay Cover Laboratory Tests

LOCATION: N.L. Industries, Pedricktown, N. J.

DATE Sampled: 8/10/83

UF&S REF. NO.: 4617

#### TEST/INSPECTION RESULTS

All tests and inspections performed on the above landfill were satisfactory and met or exceeded project qualifications.

Below are the results of the most recent tests.

Sample #	Permeability (cm/sec)	Depth <u>of Clay</u>	Location
1	$4.68 \times 10^{-8}$	12.0"	90' Upgradient apron fr. A. Headwall
2	$2.67 \times 10^{-8}$	7.0"	40' S. of N. corner Phase A
3	$3.55 \times 10^{-8}$	17ዿ''	Center gradient edge slope div. 30' fr.
4	$2.07 \times 10^{-8}$	14岁"	Gradient Div. on E. edge of side slope
5	$4.02 \times 10^{-8}$	8½''	40' E. of W. corner Phase B
6	$2.61 \times 10^{-8}$	145"	60' Upgradient fr. B. Headwall
7	$3.99 \times 10^{-8}$	8 3/4"	20' Upgradient fr. B sump wall
8	$4.15 \times 10^{-8}$	75"	20' Upgradient fr. A sump wall
9	$4.70 \times 10^{-8}$	19 3/4"	20' Upgradient fr. B gas monitoring Well #.
_			

Proctor Per A.A. - S.H.T.O. (T 180-72)
Permeability Per, "Falling Head Permeability Test" by K.H. Head in,
"Manual of Soil Laboratory Testing, Volume 2. November 1981"

#### LABORATORY TEST RESULTS

Proctor Moisture Density Relationship

Test #1 114.6 PLF Test #2 116.0 PLF

Respectfully submitted,

William R. Underwood, P.E.

3 South Black Horse Pike Mt. Ephraim, N. J. 08059

William R. Underwood, P. E.

William M. Furman, Manager

Soil Borings - Soil Engineering - Testing - Inspection - Concrete - Steel - Asphalt - Masonry

CLIENT:

Haas Construction Company

PROJECT:

N.L. Industries

REQUIREMENT:

Fill Cover Analysis, Woolwich Sand & Gravel

DATE:

July 26, 1983

UF&S REF. NO. 4580

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## LABORATORY ANALYSIS

#### SIEVE ANALYSIS

PERCENT PASSING

		ENI LW99TMG	1
SIEVE SIZE	#1 Sand SAMPLE	#2Grave1 SAMPLE	#3 Top Soil SAMPLE
			·
3/4"	100	90.0	100
1"	100	100	100
<b>#</b> 4	100	62.0	99.3
<b>#</b> 16	97.8	49.3	92.7
<b>#</b> 50	29.6	9.7	42.6
<b>#</b> 100	9.3	3.0	25.0
<b>#</b> 200	2.9	1.2	10.0
Permeabili	ity 2.5 X 10 <sup>-2</sup>	7.5 X 10 <sup>-2</sup>	1.8 x 10 <sup>-2</sup>

UNDERWOOD, FURMAN & SNYDER TESTING LABORATORIES, INC.

William R. Underwood, P.E.

With a Unt

CC to Mr. M. Corbin, Weston Way West Chester, Pa.

. 3 South Black Horse Pike Mt. Ephraim, N. J. 08059

William R. Underwood, P. E.

William M. Furman, Manager

Soil Borings - Soil Engineering - Testing - Inspection - Concrete - Steel - Asphalt - Masonry

CLIENT:

Haas Construction Co.

PROJECT:

N.L. Industries, Pedricktown, N. J.

INSPECTION REQUIRED: Topsoil & Planting Cover

DATE:

Sept. 21, 1983

UF&S REF. NO.:

4669-A

#### REPORT

#### Purpose

The purpose of this report is to address the planting and maintenance of the final cover of the solid waste landfill at the above location.

#### Planting

The tops and slopes of the above landfill were planted with mixture of fertilizer and seeds as follows:

The slopes were planted with 100 pounds of annual rye and 25 pounds of crown vetch per acre.

The top was planted with 50 pounds of perrenial rye, 30 pounds of K-31 and 30 pounds of chewing fescue per acre.

#### Fertilizing

The entire cover was fertilized with 300 pounds of 10-20-10 fertilizer per acre and one thousand pounds of lime per acre. Maintenance

After initial successful growth of the above, fertilizer, as required may be added during March of 1984.

No cuttings of the above plantings should be performed.

Any erosion of the slopes should be repaired as soon as possible.

#### Qualifications

The above information is per Lewis Colameco of Lewis Colameco & Sons of Pennsylvania (215-647-4977). Any inquiries should be directed to him.

UNDERWOOD, FURMAN & SNYDER TESTING LABORATORIES, INC.

Miam R Underwood P.E.



## inter-office memorandum

TO:

M. Corbin

DATE: 18 November 1983

FROM:

SUBJECT: N. L. INDUSTRIES

W. O. No.: 0793-08-04

Pedricktown, N.J. Landfill Facility

Secondary Liner Leak Detection System Monitoring

Sumps A & B

Steve Holt and I established a liquid level monitoring procedure and obtained the initial (certified) "sounding" measurements to liquid levels in secondary sumps A & B today. (See sketch and table of measurements included herein).

After reviewing and testing several methods, we established the following procedure for obtaining measurements to accurately monitor the liquid levels in the secondary sumps.

#### Procedure as follows:

- 1. Confirm that the 1½" PVC suction line (retained in place for secondary sump pumping) is <u>laying on the</u> invert of the 10" C.M.P.
- 2. Insert the 1/2" copper (collapsible) "sounding" rod into the 1½" PVC line as far as the wire stop at the end of the 1½" PVC line.
- 3. Using chalk (or other temporary marking material) carefully mark the 1/2" copper "sounding" rod at a point coinciding with the end of the 10" C.M.P.
- 4. Withdraw the copper "sounding" rod and measure the dimension along the rod from the point marked as described in 3 above, to the \*beginning of the wetted area of the rod.
  - \* The measurement described in 4 above must be taken promptly (particularly in dry/windy conditions) due to evaporation of moisture from the rod surface. To maximize clarity of the beginning of the wetted area, the surface of the "sounding" rod should be "chalked" in the approximate area of anticipated liquid level prior to insertion.

c-..-a:--

The following table is a record of the initial "sounding" measurements obtained at secondary sumps A & B today in accordance with the procedure described above:

Sump	Sounding Measurement	Date	<u>Time</u>	
A B	28.26' · 29.90'	November 18, 198 November 18, 198		
	-10'-P.V.C.	100	dot end .	
Prime	<u>-10 - P.V.C:</u> ory - Sump)-		1/2" &	copper-
	"-CM P. (:Secon		3000	
_Liquid_ _Livel=	· ·		77	
::LCVC1 =	Wetled length	Begin wet	Hed_	_
-Wire sto	ap \znserfcop			

The procedure described above is relatively simple and will insure maximum consistency in monitoring the liquid levels.

It is my understanding that the secondary sumps will be monitored regularly, every Monday A.M., prior to commencement of leachate pumping from the primary sumps for that week.

SKETCH SHOWING DETAILS OF SECONDARY SUMP SOUNDINGS

If you have any questions or need additional information, don't hesitate to call.

· Dick

DEC 12 3 00 ch . 83

WASTE HANAGEMENT

December 14, 1983

Mr. Edward J. Londres
Assistant Director - Engineering
Division of Waste Management
32 E. Hanover Street
Trenton, NJ 08625

Re: Landfill Closure Certification
NL Industries, Inc.
Pedricktown, New Jersey
Facility No. 1706 C (EPA I.D. No.: NJD0618432)

#### Dear Mr. Londres:

Enclosed are: (i) the certification of closure of the referenced facility by Roy F. Westin, Inc., a licensed New Jersey engineer, and (ii) a topograhic as built survey prepared by Albert A. Fralinger, Jr., P.A., a licensed New Jersey surveyor. Based on the enclosed certification and survey, NL certifies to NJDEP that the referenced landfill was closed in accordance with the specifications of the closure plan approved by NJDEP as set forth in the following documents:

- A. The Engineering Design Approval, dated April 5, 1978 as amended.
- B. Closure/Post Closure Plans submitted December 15, 1982.
- C. Addendum to the Closure/Post Closure plans submitted March 24, 1983.
- D. Design Clarification report submitted August 16, 1983.

All documents, plans and drawings required by NSNJ to discharge its post-closure obligations will be provided by NL to NSNJ. In addition, NSNJ has been present during closure of the landfill and has been provided with information on an ongoing basis to provide for a smooth transition.

DWE10 I
NL Industries, Inc.
1230 Avenue of the Americas, New York, N.Y. 10020



WESTON WAY WEST CHESTER, PA. 19380 PHONE: (215) 692-3030 TELEX: 83-5348

#### 13 December 1983

Mr. Edward J. Londres
Assistant Director-Engineering
Division of Waste Management
N.J. Department of Environmental Protection
32 E. Hanover Street CN027
Trenton, NJ 08625

Reference: Landfill Closure Certification

N.L. Industries, Inc. Pedricktown, New Jersey Facility No. 1706C EPA I.D. #NJD061843249

Dear Mr. Londres:

This is to certify that the closure of the landfill referenced above has been completed by Haas Construction Inc. (prime contractor) in accordance with the following closure plan documents:

- Closure/Post-Closure Plan dated 12/13/82, submitted 12/15/82, prepared by WESTON.
- Addendum report to Closure/Post-Closure Plan dated 3/22/83, submitted 3/24/83, prepared by WESTON in response to NJDEP comments on 12/13/82 Closure/Post-Closure Plan.
- 3. Design clarification letter report dated 8/11/83, submitted 8/16/83, prepared by WESTON. and
- 4. Engineering Design Approval (NJDEP document) dated 4/5/78.

In accordance with the October 27, 1983 meeting between N.L. Industries, WESTON and NJDEP, WESTON provides the following certification for the landfill closure work:

1. A clay cap of a minimum thickness of 6 inches with clay having a maximum permeability of lx10-7 centimeters per second has been placed over the entire landfill surface and has been compacted to at least 90% as determined by the ASTM D698 method. The attached (Attachment #1) certified laboratory results



Mr. Edward J. Londres -2-N.J. Department of Environmental Protection 13 December 1983

provided by the Contractor (U.F.&S Reference No. 4617) provides the results of the permeability testing of nine clay samples and indicates thickness of the clay cap at these locations.

- 2. A final cover system consisting of a gravelly sand and sand flow/fill zone and a 6 inch top soil layer was installed and the flow zone meets or exceeds the permeability standards set forth in the closure plan documents. The certified analysis (provided by the Contractor, U.F.&S Reference No. 4580) of the cover system materials is included as Attachment #2.
- 3. The final elevation of the landfill does not exceed 45 feet above mean sea level.
- 4. The side slopes of the landfill are within the limits established by the closure plan documents.
- 5. The final gradient of the completed final cover on the top area of the landfill was measured to be a minimum of 1%.
- 6. Drainage structures including retaining walls around the leachate collection sumps and slope drains were installed and completed in accordance with the closure plan documents.
- 7. Seeding of the final cover was performed and completed in accordance with the specifications. The application rates were established by the Contractor and provided in Attachment \$3 (U.F.&S Reference No. 4669-A). The slopes were planted with 100 lbs. of annual rye and 25 lbs. of crown vetch per acre. The top was planted with 50 lbs. of perennial rye, 30 lbs. of Kentucky 31 and 30 lbs. of chewing fescue per acre. Vegetative cover was established exclusive of some areas which may require reseeding in the spring under the post closure plan.
- 8. An 8 foot high chain link security fence has been installed surrounding the landfill.
- 9. Attachment #4, Memorandum from R. Buss to M. Corbin dated November 18, 1983 details the procedure for sounding of the liquid levels in the monitoring sumps in accordance with our meeting of October 27, 1983 and

WESTERN

Mr. Edward J. Londres -3-N.J. Department of Environmental Protection 13 December 1983

provides the initial certified sounding measurements from the top of the riser pipes to the liquid levels as recorded on November 18, 1983. A sounding measurement of 28.3 feet was recorded in Sump A and a sounding measurement of 29.9 feet recorded in Sump B.

If WESTON can provide additional information on the closure work performed at the Pedricktown Plant Landfill please contact the undersigned.

Very truly yours,

ROY F. WESTON, INC.

Michael H. Corbin, P.E.

Project Manager

Dames H. Dougherty, P.E.

Vice President

MHC/JHD/snh

Attachments

cc: Paul Kahn, Esq.

Keith Ornsdorff, Esq.

Mr. Roger Ennis, Evir. Eng. (NJDEP)

Jeffrey Jacobs, Esq., N.L.

Mr. Dean Ervin, N.L.

Mr. Stephen Holt, N.L.

3 South Black Horse Pike Mt. Ephraim, N. J. 08059

William R. Underwood, P. E.

William M. Furman, Ma

Soil Borings - Soil Engineering - Testing - Inspection - Concrete - Steel - Asphalt - Masonry			
CLIENT:	Haas Construction Company		
PROJECT:	N.L. Industries		
TEST/INSPECTION REQUIRED:	Clay Cover Laboratory Tests		
LOCATION:	N.L. Industries, Pedricktown, N. J.		
DATE Sampled:	8/10/83		
UF&S REF. NO.:	4617		

#### TEST/INSPECTION RESULTS

All tests and inspections performed on the above landfill were satisfactory and met or exceeded project qualifications.

Below are the results of the most recent tests.

Sample #	Permeability (cm/sec)	Depth of Clay	Location
1	$4.68 \times 10^{-8}$	12.0"	90' Upgradient apron fr. A. Headwa
2	$2.67 \times 10^{-8}$	7.0"	40' S. of N. corner Phase A
3	$3.55 \times 10^{-8}$	17월"	Center gradient edge slope div. 30
4	$2.07 \times 10^{-8}$	14날"	Gradient Div. on E. edge of side s
5	$4.02 \times 10^{-8}$	8놏"	40' E. of W. corner Phase B
<b>6</b> ·	$2.61 \times 10^{-8}$	14%"	60' Upgradient fr. B. Headwall
7	$3.99 \times 10^{-8}$	. 8 3/4"	20' Upgradient fr. B sump wall
8	$4.15 \times 10^{-8}$	·7눌"	20' Upgradient fr. A sump wall
9	4.70 x 10 <sup>-8</sup>	19 3/4"	20' Upgradient fr. B gas monitoring Well 180-72)

Proctor Per A.A. - S.H.T.O. (T 180-72)
Permeability Per, "Falling Head Permeability Test" by K.H. Head in,
"Manual of Soil Laboratory Testing, Volume 2. November 1981"

### LABORATORY TEST RESULTS

Proctor Moisture Density Relationship

Test #1 114.6 PLF Test #2 116.0 PLF

Respectfully submitted,

William R. Underwood, P.E.

3 South Black Horse Pike Mt. Ephraim, N. J. 05059

William R. Underwood, P. E.

William M. Furman.

Soil Borings - Soil Engineering - Testing - Inspection - Concrete - Steel - Asphalt - Masonry

CLIENT:

Haas Construction Co.

PROJECT:

N.L. Industries, Pedricktown, N. J.

INSPECTION REQUIRED: Topsoil & Planting Cover

DATE:

Sept. 21, 1983

UF&S REF. NO.:

4669-A

#### REPORT

#### Purpose

The purpose of this report is to address the planting and maintenance of the final cover of the solid waste landfill at the above location.

#### Planting

The tops and slopes of the above landfill were planted with mixture of fertilizer and seeds as follows:

The slopes were planted with 100 pounds of annual rye and 25 pounds of crown vetch per acre.

The top was planted with 50 pounds of perrenial rye, 30 pounds of K-31 and 30 pounds of chewing fescue per acre.

## Fertilizing

The entire cover was fertilized with 300 pounds of 10-20-10 fertilizer per acre and one thousand pounds of lime per acre. Maintenance

After initial successful growth of the above, fertilizer, as required may be added during March of 1984.

. No cuttings of the above plantings should be performed.

Any erosion of the slopes should be repaired as soon as possible.

### Qualifications

The above information is per Lewis Colameco of Lewis Colameco & Sons of Pennsylvania (215-647-4977). Any inquiries should be: directed to him.

UNDERWOOD, FURMAN & SNYDER TESTING LABORATORIES, INC.

A Harwood

0204 001 NLI

December 14, 1983 Page 2

The landfill, as noted above, has been closed in accordance with the specified documents. Since closure, NL Industries has been maintaining the facility in accordance with Post Closure plans submitted December 15, 1982. Post closure activities, including maintenance and monitoring of the landfill's condition, are now the responsibility of National Smelting of New Jersey (NSNJ) as described in Amendment to Administrative Consent Order dated February 7, 1983. This responsibility specifically includes annual analysis of top soils, reseeding and fertilization of the top cover as recommended by the County Agricultural Agent, pumping of leachate, and the filing of reports as required by State and Federal agencies.

Respectfully submitted,

Steven W. Holt Project Manager Dean W. Ervin

Director, Metals Operations

DWE/IW

enclosure



## inter-office memorandum

TO:

M. Corbin

DATE: 18 November 19

FROM:

SUBJECT: N. L. INDUSTRIES

W. O. No.: 0793-08-04

Pedricktown, N.J. Landfill Facility

Secondary Liner Leak Detection System Monitoring

Sumps A & B

Steve Holt and I established a liquid level monitoring procedure and obtained the initial (certified) "sounding measurements to liquid levels in secondary sumps A & B today. (See sketch and table of measurements included herein).

After reviewing and testing several methods, we established the following procedure for obtaining measurements to accurately monitor the liquid levels in the secondary sumps.

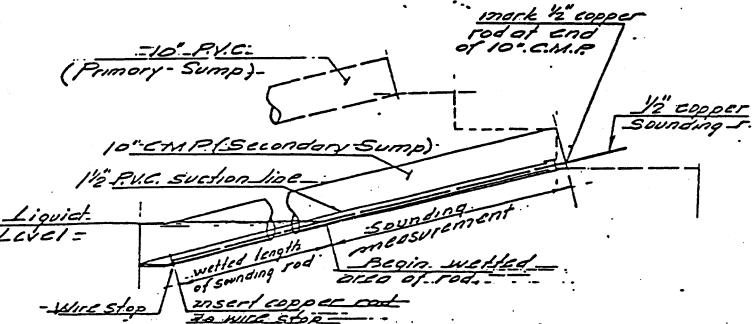
#### Procedure as follows:

- 1. Confirm that the 1½" PVC suction line (retained in place for secondary sump pumping) is laying on the invert of the 10" C.M.P.
- 2. Insert the 1/2" copper (collapsible) "sounding" rod into the 1½" PVC line as far as the wire stop at the end of the 1½" PVC line.
- 3. Using chalk (or other temporary marking material) carefully mark the 1/2" copper "sounding" rod at a point coinciding with the end of the 10" C.M.P.
- 4. Withdraw the copper "sounding" rod and measure the dimension along the rod from the point marked as described in 3 above, to the \*beginning of the wetted area of the rod.
  - \* The measurement described in 4 above must be taken promptly (particularly in dry/windy conditions) due to evaporation of moisture from the rod surface. To maximize clarity of the beginning of the wetted area, the surface of the "sounding" rod should be "chalked" in the approximate area of anticipated liquid level prior to insertion.

The following table is a record of the initial "sounding" measurements obtained at secondary sumps A & B today in accordance with the procedure described above:

Corbin

Sump	Sounding Measurement	Date Time	
A B	28.26' 29.90'	November 18, 1983 / 1:30 p.m November 18, 1983 / 1:45 p.m	
		· · · · · · · · · · · · · · · · · · ·	



#### SKETCH SHOWING DETAILS OF SECONDARY SUMP SOUNDINGS

The procedure described above is relatively simple and will insure maximum consistency in monitoring the liquid levels.

It is my understanding that the secondary sumps will be monitored regularly, every Monday A.M., prior to commencement of leachate pumping from the primary sumps for that week.

If you have any questions or need additional information, don't hesitate to call.

· Vick

N

RECEIVED FEB 2 3 1984

ENVIRONMENTAL CONTROL

February 16, 1984

Mr. Frank Coolick, Chief
Bureau of Hazardous Waste Engineering
Division of Waste Management
N.J. Department of Environmental Protection
32 E. Hanover St.
Trenton, N.J. 08625

Dear Mr. Coolick:

Your letter of January 23rd refers to the observance of localized cover soil erosion and loss of vegetative cover, and minor wash outs of slopes during a field inspection of the referenced landfill on December 7, 1983 to support a determination that closure is "deemed" incomplete.

The field inspection of December 7 is not relevant to the issue of closure as it was carried out more than two months following the completion of closure of the landfill which took place during the week of September 19, 1983. Closure of the landfill was completed on the verbal authorization of the Department since the time needed by the Department to formally issue its written approval of NL's closure and post closure plans would have delayed closure for an additional year. The formal written approval was issued by the Department on October 20, 1983, a month after closure had been completed in accordance with the Department's verbal authorization.

The localized erosion and loss of vegetative cover was caused by heavy rainfall after the completion of closure. Prior to these rains, vegetative cover had been established over the entire landfill surface. Furthermore, while vegetative cover is necessary for the long term protection of the clay cap, it is the clay cap itself which seals the landfill; there are no indications that the clay cap has been damaged.

The closure and post closure plans approved by the Department contemplated localized erosion and loss of vegetative cover such as that which has been experienced at the landfill for a number of years, until vegetation is "firmly established". The post closure plan dated December 15, 1982, states:

NL Industries, Inc.
1230 Avenue of the Americas, New York, N.Y. 10020

"Until the vegetative cover is firmly established it may be expected that some erosion could take place. Soil will be replaced in these areas and they will be reseeded as necessary".

An effort was made in the fall to repair the eroded area as provided for in the post closure plan. However, due to the onset of winter, vegetation was not reestablished. We assure you that additional repairs, reseeding and revegetation of the eroded areas will be performed in the Spring.

Roy F. Weston, Inc. is writing to you under separate cover to clarify its closure certification of December 13, 1983, as to the time of completion of closure of the landfill and the presence of vegetative cover on all side slopes.

In light of the foregoing, NL requests that the Bureau reconsider its position and confirm the Department's determination that closure of the landfill was completed in September, 1983.

With respect to the three enumerated items set forth in your letter please be advised:

- 1. NL will request the surveyor to indicate headwalls, sumps and wells as you have requested. The clay core sampling locations were not surveyed, but they are described in Weston's December 13, 1983 letter to Mr. Londres and are shown in the attached sketch by Mr. Underwood (a professional engineer) of Underwood, Furman & Snyder Testing Laboratories, the laboratory which collected the samples.
- 2. Attachment 2 of Weston's December 13, 1983 letter presents the permeability results of the top soil and gravel samples performed by Underwood, Furman & Synder Testing Laboratories. The use of gravelly sand for the flow zone was included in Weston's August 11, 1983 letter supplementing the closure plan which NL submitted to the Department on August 16, 1983. The results confirming permeability of material denoted as gravel in the Underwood test report refer to this gravelly sand.
- 3. The measurement referred to in certification 7 was intended to provide a reliable reference point for future

measurements (not a comparison with other measurements previously taken) pursuant to an agreement reached with the Department on October 27, 1983. Accordingly, the results of the two procedures are not directly comparable. However, the following comparison can be made using the liquid levels measured in the secondary sumps on November 14 and November 21, 1983 in view of the consistent measurements previously obtained.

<u>Date</u>	Phase A Secondary Sump	Phase B Secondary Sump	Comments
11/14/83	1.75	3.75	Wetted length of riser pipe
11/21/83	28.3	29 <b>.</b> 9	Dry length of riser pipe
	30.05	33.65	Total length of riser pipe -

Therefore, the approximate corresponding depths to water (dry length of riser pipe) for previous measurements may be obtained by subtracting the wetted length of the sounding rod from the total length of the riser pipe. A copy of Attachment 4 to Weston's December 13 Certification of Landfill Closure, describing the procedure is enclosed for your convenience.

Sincerely

Dean W. Ervin

Director Metals Operations

DWE/gs

Enclosures

c: Paul Kahn, Esq. - w/encls.

bc:

W. Bronner

J. Jacobs

R. Losey

W. Weddendorf



WESTON WAY WEST CHESTER, PA. 19380 PHONE: (215) 692-3030 TELEX: 83-5348 RECEIVED

FEB 21 1984

ENVIRONMENTAL CONTROL

16 February 1984

Mr. Edward J. Londres
Assistant Director - Engineering
Division of Waste Management
N.J. Department of Environmental Protection
32 E. Hanover Street CN027
Trenton, NJ 08625

Reference: Landfill Closure Certification

N.L. Industries, Inc.
Pedricktown, New Jersey
Facility No. 1706C
EPA I.D. #NJD061843249

Dear Mr. Londres:

In response to the question of certification relating to final vegetative cover for the referenced facility, the following clarification to our closure certification of December 13, 1983, is provided. The certifications set-forth in our December 13 letter with the exception of the last sentence in certification paragraph #7 and certification paragraph #9 were made as of September 30, 1983, when closure of the landfill was completed.

After seeding of the final cover during the week of September 12, 1983, vegetative cover was established on all slopes. After vegetative cover was established, several heavy rain storms occurred at the site. These storms caused some localized erosion on the landfill slopes. Steps were undertaken to repair the localized erosion using maintenance procedures as set forth in the post closure plan. The last sentence in our certification paragraph #7 notes that localized erosion was observed on November 18, 1983, at the time of the sump measurements subsequent to landfill closure.



TO: Mr. Edward J. Londres -2-N.J. Dept. of Environmental Protection

16 February 1984

Sump sounding measurements detailed in our certification paragraph #9 were taken on November 18, 1983, pursuant to our October 27, 1983, meeting with NJDEP. This sounding certification provides the basis for future monitoring of liquid levels in these sumps under the post closure plan.

If I can provide further information on this matter, please contact me.

Sincerely,

ROY F. WESTON, INC.

Michael H. Corbin Project Manager

MHC: jac

cc: Dean Ervin - NL Industries
 Jeffrey Jacobs - NL Industries
 William Weddendorf - NL Industries
 James Dougherty - WESTON

3 South Black Horse Pike Mt. Ephraim, N. J. 08059

William R. Underwood, P. E.

William M. Furman, Manager

Soil Borings - Soil Engineering - Testing - Inspection - Concrete - Steel - Asphalt - Masonry

CLIENT:

Haas Construction Company

PROJECT:

N.L. Industries

TEST/INSPECTION REQUIRED: Clay Cover Laboratory Tests

LOCATION:

N.L. Industries, Pedricktown, New Jersey

DATE:

Sampled-8/10/83

UF&S REF. NO.:

4617

#### TEST/INSPECTION RESULTS

All tests and inspections performed on the above landfill were satisfactory and met or exceeded project qualifications.

Below are the results of the most recent tests.

Sample #	Permeability ( cm/sec)	Depth of Clay (")	Location
1 2 3 4 5 6 7 8	4.68 x 10 <sup>-8</sup> 2.67 x 10 <sup>-8</sup> 3.55 x 10 <sup>-8</sup> 2.07 x 10 <sup>-8</sup> 4.02 x 10 <sup>-8</sup> 2.61 x 10 <sup>-8</sup> 3.99 x 10 <sup>-8</sup> 4.15 x 10 <sup>-8</sup> 4.70 x 10 <sup>-8</sup>	7.0" 4 17½" 6 14½" 6 8½" 4 14½" 6 8 3/4" 2 7½" 2	O' Upgradient apron fr. A Headwall O' S. of N. corner Phase A Senter gradient edge slope div.30' fr Gradient Div. on E. edge of side slope O' E. of W. corner Phase B O' Upgradient fr. B Headwall O' Upgradient fr. B sump wall O' Upgradient fr. A. sump wall O' Upgradient fr. B. gas monitoring Wall #2

Proctor Per A.A. - S.H.T.O. (T 180-72)

Permeability Per, "Falling Head Permeability Test" by K.H. Head in, "Manual of Soil Laboratory Testing, Volume 2. November 1981"

#### LABORATORY TEST RESULTS

Proctor Moisture Density Relationship

Test. 11

114.6 PLF

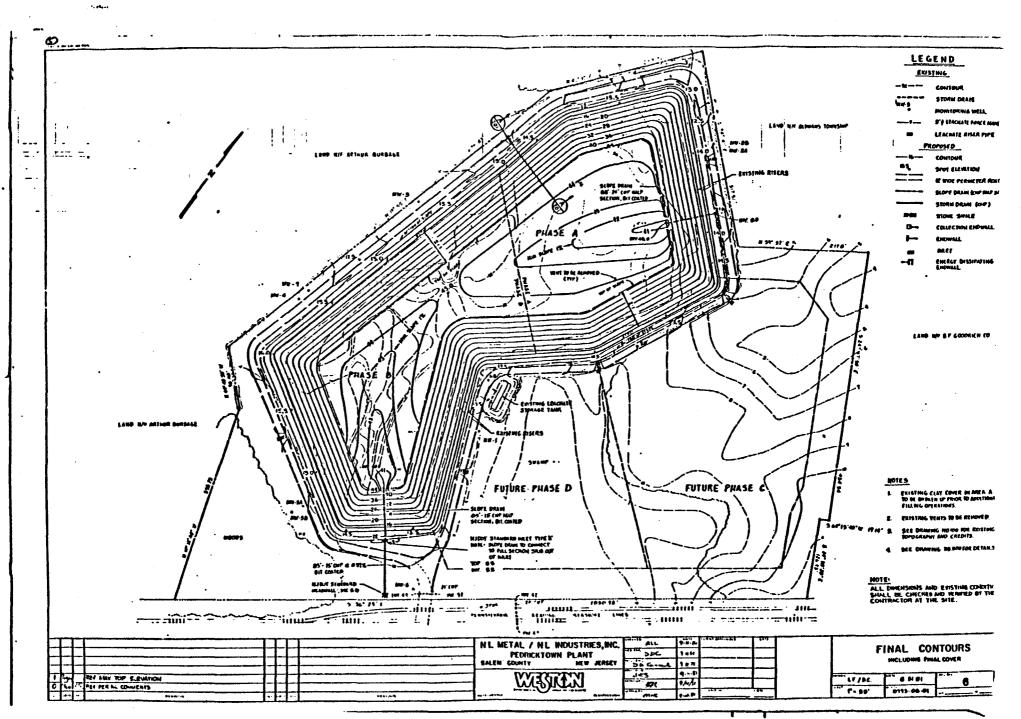
Test #2

116.0 PLF

Respectfully submitted.

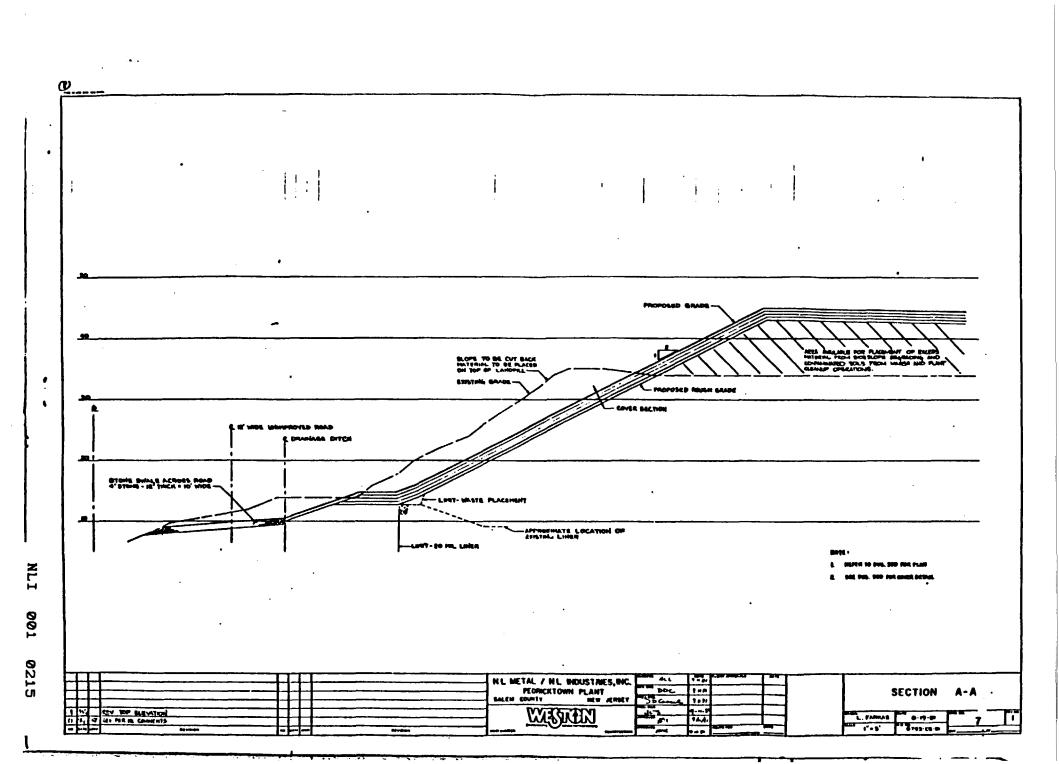
William R. Underwood, P.E.

0213 001 NLI

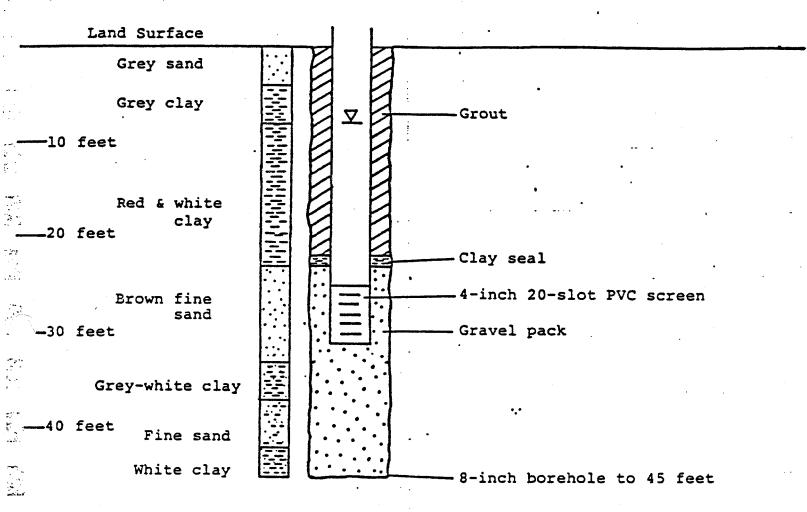


NLI 001

0214



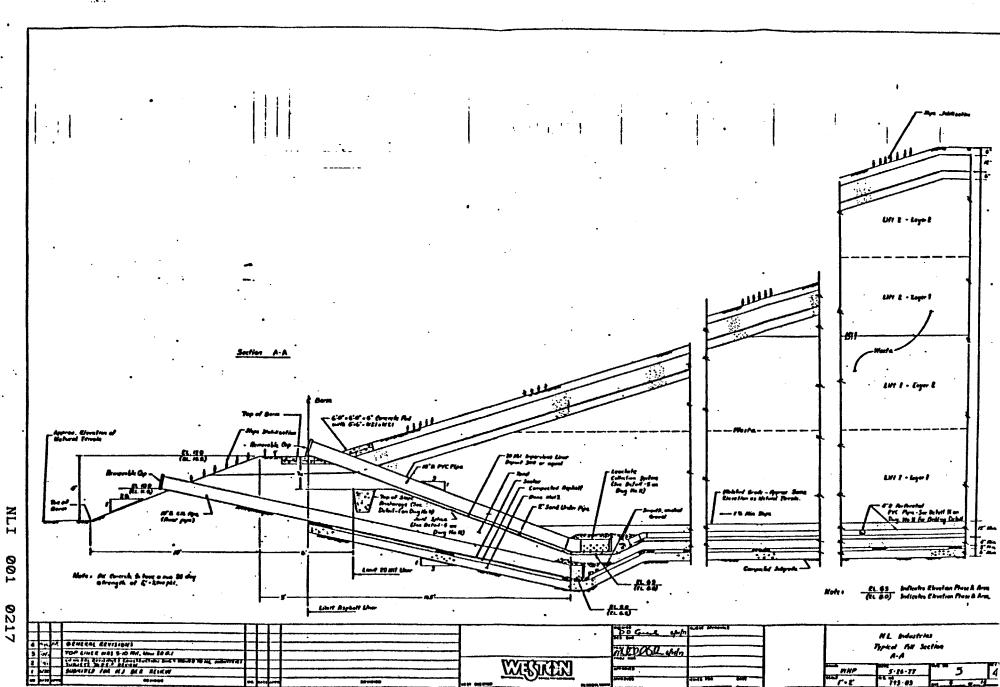
N/L-INDUSTRIES, INC.
Pedricktown, New Jersey
Completion Diagram for Well\_CR2

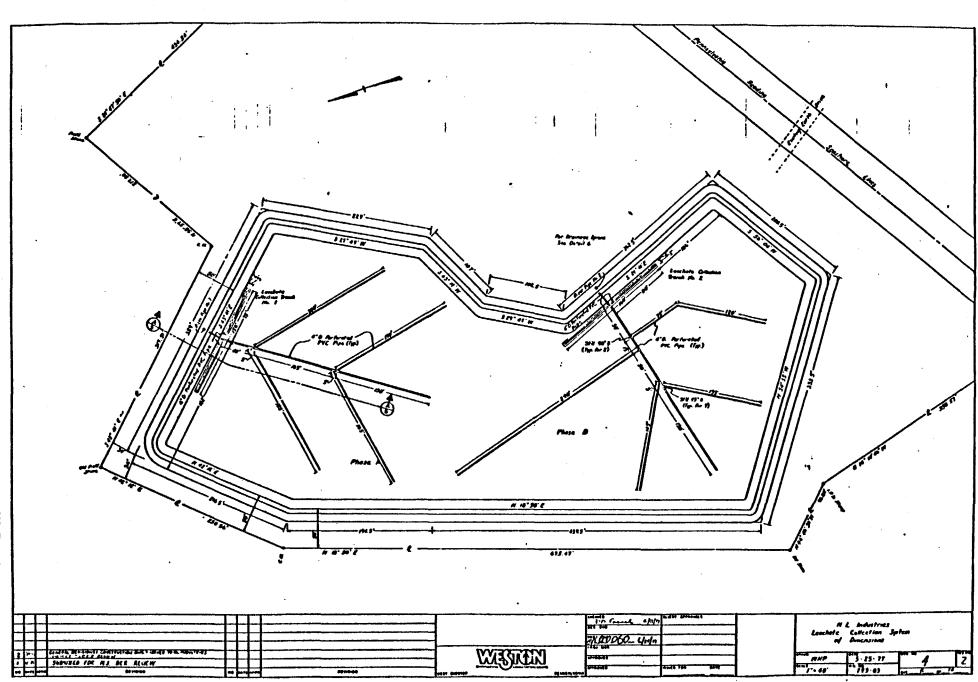


Well completed March 29 1980

▼ SWL = 7.9 feet below grade, April 3 1980

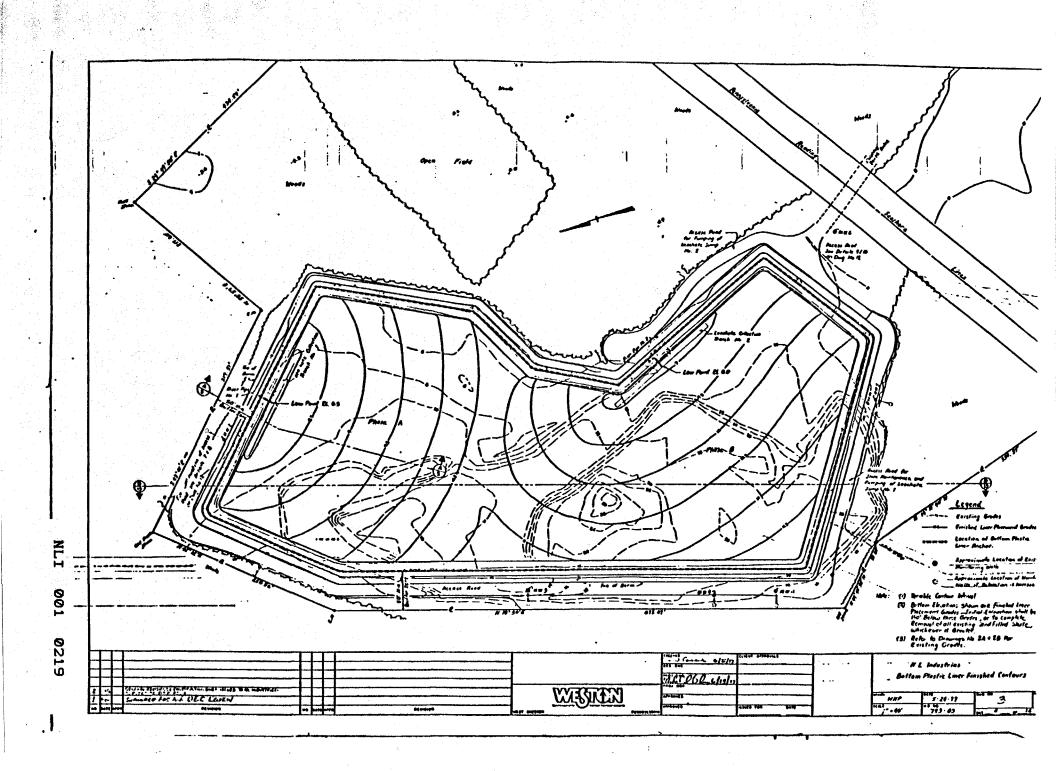
Leggette, Brashears & Graham, Inc. April 1980





NLI 001

1 0218



# EXHIBIT B MONITORING WELL LOGS

### WELL LUG

LEGGETTE, BRASHEARS & GRAHAM, INC.

CONSULTING GROUND-WATER GEOLOGISTS

Z2 - DANBURY ROAD --WILTON, CT. D6897 THICK-METHO SPEETS . Pedricktown, NJ Topsoil BR 25 Sand, brown, medium to coarse 26 April 1, 1980 1 Sandy clay 27 DRILLING A. C. Schultes & Sons 11 Sand, brown, coarse 38 Matter rotary Clay, sandy, tan 45 Marmon Ditch Examines The Grubman REFERENCE POINT Grade ELEVATION OF R. P. SCREDI-PVC Dum. 4-inch No 0.020 i 31-37 PUMPING TESTS OURATION. STATIC WATER 4.1 feet\* PUMPING WATER >15 qpm REMARKS \* below top of 4-inch PVC casing 0221 001 NLI

WELL LUS

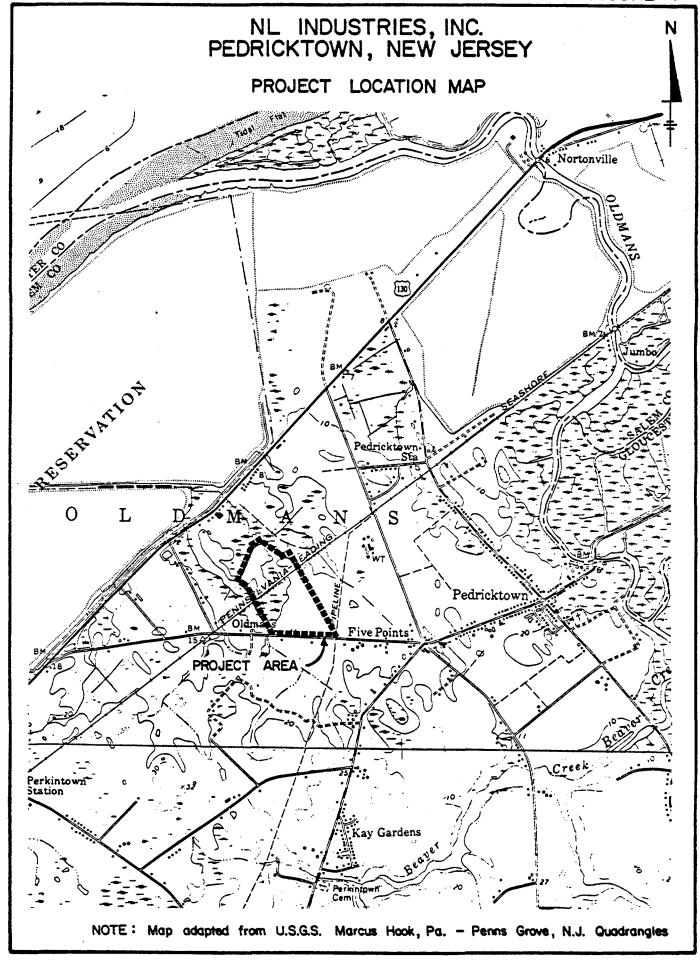
LEGGETTE, BRASHEARS & GRAHAM, INC.

CONSULTING GROUND-WATER GEOLOGISTS

72 DANBURY ROAD

WILTON, CT. 06897

WILTON, C	T. 06	897	- PAGE 1 OF 1 PAGES
BENCRIPTION	THICK. MEMO FEETI	SEPTI :	N/L Industries, Inc.
Sand, brown, medium to fine, trace			Locamon Pedricktown, NJ
medium to fine gravel and silt	16	16	was Me A (Replacement).
Sand, brown, medium to coarse, some	•	•	Date December 12-13, 1979
medium to fine gravel, trace silt	3	19	Daniline Craig Testing Labs, Inc
Sand, Tan, fine to coarse, some			Directing Mud rotary (bentonite)
medium to fine gravel. trace silt	8	27	SAMPLINEDitch & 1 split spoon
Sand, tan, fine to medium and clay,			SAMPLES Lee N. Grubman
white, sandy	2	29	REFERENCE Land surface
Sand, tan-brown, fine-to medium	114	3.03-	ELEVATION OF R. P.
Clay, red-brown, with sandy clay			CASING
<u>streaks</u>	ц	343	Scatter Slotted PVC
			Dum. 4 inches stor No. 20
			******* 2.5-32.5 feet
			PUMPING TESTS DATE
···			DUSATION
· · · · · · · · · · · · · · · · · · ·			3.0 feet(12/17/79)
			Pumping WATER
			Livia
			during development
			REMARKS
			* Replacement ±10 feet
			east of original well A now destroyed.
			. :
	1		
•	1	-	
	+		NLI 001 0222

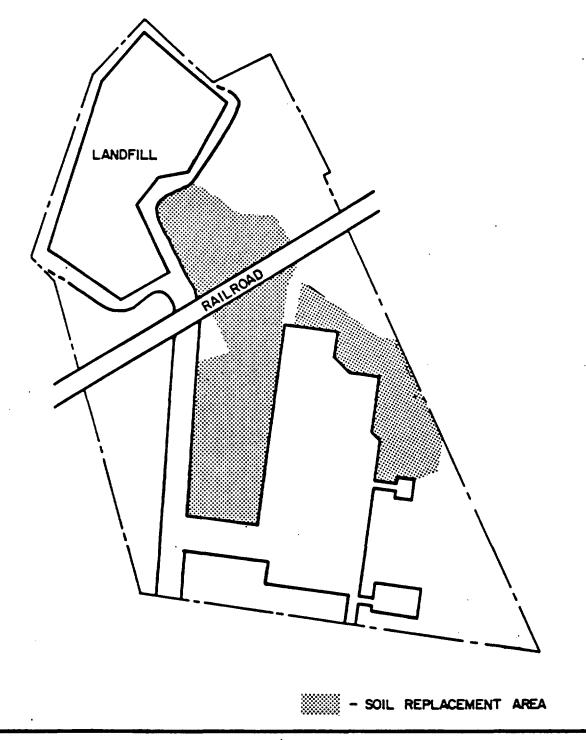


N



SOIL REPLACEMENT AREA

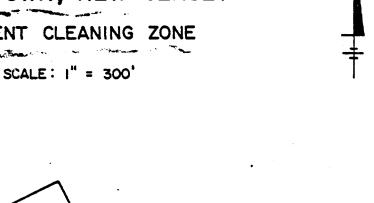
SCALE: I" = 300'

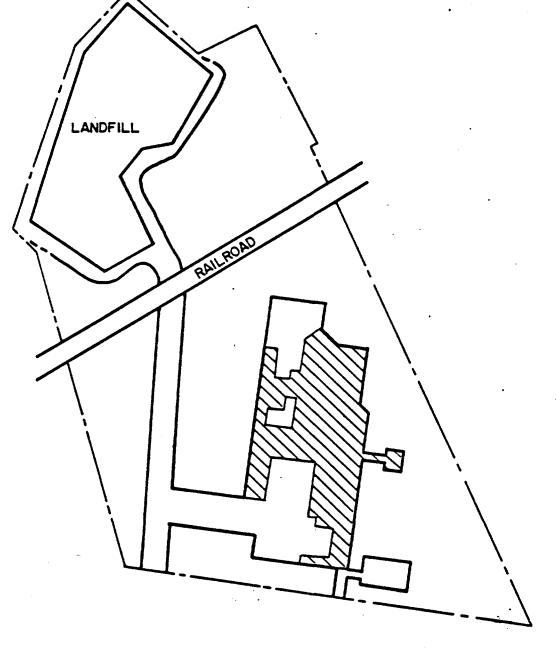


N

# NL INDUSTRIES, INC. PEDRICKTOWN, NEW JERSEY

\*PAVEMENT CLEANING ZONE





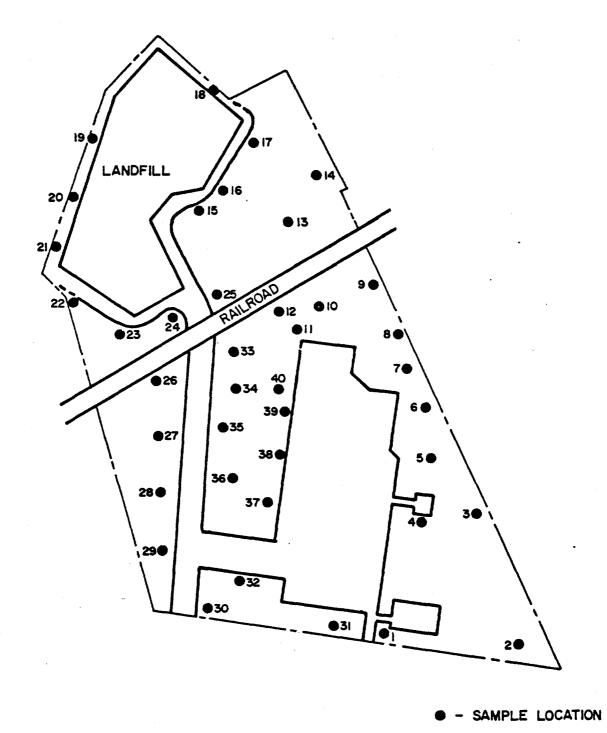
./// - PAVEMENT CLEANING ZONE

NL INDUSTRIES, INC. PEDRICKTOWN, NEW JERSEY

SOIL SAMPLING LOCATIONS (1980)

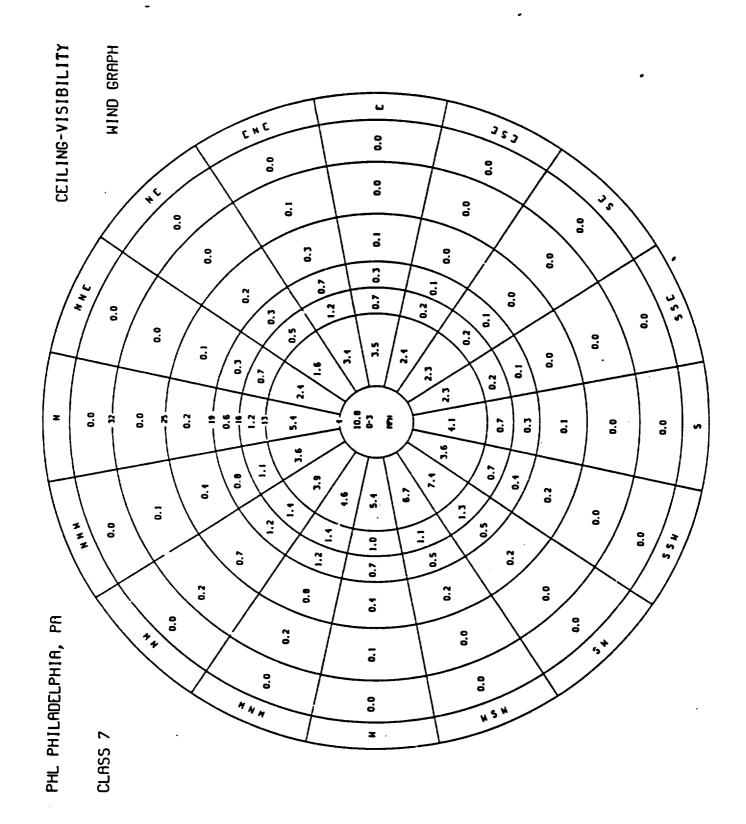
SCALE: 1" = 300'





APPROXIMATE SOIL SAMPLE LOCATION (1980)

FIGURE 9 - WIND ROSE FOR PHILADELPHIA, PA



## **Exhibits**



# EXHIBIT A LANDFILL CLOSURE INFORMATION

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ir Car

ر نست

### **NSNJ PEDRICKTOWN SITE**

### CHRONOLOGY OF EVENTS RELATIVE TO ON-SITE LANDFILL

August 1982 Plant Soils and Marsh Soils Cleanup Activities Started.

December 1982 Started Placement of Landfill Clay Cap to Reduce Leachate

Generation.

August 1983 Clay Cap Completed and Tested August 10, 1983.

October 20, 1983 Closure Plan Approved by NJDEP.

December 15, 1983 Closure Certified by NL Industries, Inc. and Roy F. Weston,

Inc.

December 22, 1983 Landfill Maintenance Operations Transferred to NSNJ.

May 29, 1984 NL Repairs Winter Damage to Landfill Cover.

June 15, 1984 National Bank of Georgia Terminates Final Employee.

June 18, 1984 NL Voluntarily Enters Site to Maintain Landfill.

January 25, 1985 Phase B - Automatic Operation Commences.

February 20, 1985 Phase A - Automatic Operation Commences.

6:9



## State of Rem Jerney

### DEPARTMENT OF ENVIRONMENTAL PROTECTION

DIVISION OF WASTE MANAGEMENT 32 E. Hanover St., CN 027, Trenton, N.J. 08625

DR. MARWAN M. SADAT

...

j.,

October 20, 1983

LING F PEREIRA

Mr. Stephen W. Holt N. L. Industries, Inc. Penns Grove - Pedricktown Road Pedricktown, New Jersey 08067

RE: N. L. Industries, Inc., Oldmans Township, Salem County, New Jersey Facility Registration Number 1706C

Dear Mr. Holt:

The Bureau of Hazardous Waste Engineering has reviewed the submittals from Roy F. Weston, Inc. made on December 15, 1982, March 24, 1983 and August 16, 1983 in behalf of N. L. Industries, Inc. in reference to the Closure and Post-Closure plans for a hazardous waste landfill located at the former N. L. Industries, Inc. Pedricktown plant premises. This submittal was made in response to items 2(d) and 3 of the Administrative Consent Order signed by N. L. Industries, Inc. and the Department on October 6, 1982 regarding the final closure of the existing hazardous waste landfill.

The Bureau of Hazardous Waste Engineering has found said submittals in conformance with regulatory standards set forth in Subchapters 9 and 11 of N.J.A.C. 7:26 and hereby approves the closure and post-closure plans subject to compliance with the following conditions:

- 1) The closure shall follow specifications of the engineering designs and narrative prepared by Roy F. Weston, Incorporated; specifically: the engineering report prepared by James H. Dougherty, P.E. and Amir A. Metry, P.E. dated December 13, 1982, the engineering report in response to NJDEP comments prepared by Michael H. Corbin, P.E., dated March 22, 1983, drawing sheet No. 5 Rev. No. 5 prepared by Roy F. Weston, Inc. dated June 24, 1977, drawing sheet No. 6 Rev. No. 3 prepared by Roy F. Weston, Inc. dated September 11, 1981, drawing sheet No. 7 Rev. No. 3 prepared by Roy F. Weston, Inc. dated September 11, 1981 and drawing sheet No. 8 Rev. No. 1 prepared by Roy F. Weston, Inc. dated September 11, 1981, except as modified herein.
- 2) The landfill shall be graded in accordance with the narratives prepared by Roy F. Weston, Inc. dated December 13, 1982 and as shown on drawing sheet No. 5 Rev. No. 5 dated June 24, 1977, prepared by Roy F. Weston, Inc.

- After grading, a clay cap of a minimum thickness of six (6) inches and maximum permeability of 1 x 10 cm/sec shall be placed over the entire landfill surface and compacted to 90% minimum as per ASIM D-1557 Method C. This item is expected to be completed before October 30, 1983 and must be certified by a New Jersey Licensed Professional Engineer upon implementation.
  - 4) A final cover system, designed to intercept and divert any infiltration away from the clay cap layer, shall be placed on top of the clay cap. The bottom layer of this cover system shall consist of a 6-inch layer of a coarse to fine sand with a permeability of 1 x 10<sup>-2</sup> to 1 x 10<sup>-3</sup> cm/sec. Six inches of clean earth shall be placed over this bottom layer followed by six inches of top soil. These soil layers shall serve to support a vegetative cover.

The final elevation of the landfill shall be approximately 40 feet above MSL (mean sea level). The final landfill contours and drainage plan shall be as shown on drawing sheet No. 6 - Rev. No. 3 prepared by Roy F. Weston, Inc., dated September 11, 1981. The final side slopes shall be as described in the narratives prepared by Michael H. Corbin, P.E. (design clarification report) dated August 11, 1983. The side slopes on the western and southern boundaries of the landfill shall not be steeper than 2:1 (horizontal to vertical), ranging from approximately 4:1 to 2 1/2:1. Other slopes on the eastern and northern boundaries shall be 2:1 or less steep. This item is expected to be completed by October 30, 1983 and must be certified by a New Jersey Licensed Professional Engineer upon implementation.

5) The top of the landfill shall be graded at a minimum slope of 1 percent to provide positive drainage.

Surface run-off in Phase A shall be directed to a corrugated metal half-section pipe drain down the side slope as shown on drawing sheet No. 6 - Rev. No. 3 prepared by Roy F. Weston, Inc. and dated

September 11, 1981. Run-off shall then be conveyed by storm sewer or stone-lined channels to existing waterways. Surface run-off from Phase B shall be directed to a stone-lined swale down the side slope as shown on said drawing sheet, and then across the perimeter road to surface discharge. Diversions shall be constructed around the leachate sumps' riser pipes pumping pads. This item is expected to be completed by October 30, 1983 and must be certified by a New Jersey Licensed Professional Engineer upon implementation.

6) A drainage swale shall be constructed along the landfill perimeter to collect run-off from the side slopes. This storm-water shall be dissipated across the perimeter road by stone swales at the natural low-points. Design details for this drainage system shall be as shown on drawing sheet No. 8 - Rev. No. 1 prepared by Roy F. Weston, Inc. dated September 11, 1981. Completion of this item is expected by October 30, 1983 and must be certified by a New Jersey Licensed Professional Engineer upon implementation.

July di

- 7) The top soil shall be stabilized to prevent erosion by establishing a vegetative cover. This vegetative cover shall be established by spreading a type A seed mixture, and shall be mulched with straw, in accordance with the New Jersey Department of Transportation (N.J.D.O.T.) specifications. The side slopes shall be seeded with N.J.D.O.T. type E seed mixture, which includes crown vetch, and shall be mulched to minimize the potential of soil erosion until vegetation is established. Completion of this item is expected by October 30, 1983 and must be certified by a New Jersey Licensed Professional Engineer upon implementation.
- 8) A permanent fence shall be installed around the landfill and all monitoring wells. The fence shall be at least 8-feet high and topped with three strands of barbed wire. Vehicle access gates with locks shall be installed at the landfill entrance read and directly north of the leachate storage tank. Completion of this item is expected by October 30, 1983 and must be certified by a New Jersey Licensed Professional Engineer upon completion.
- 9) Liquids which have accumulated in the leak detection sumps of both Phase A and Phase B shall be removed, to restore these sumps to effective monitoring points for assessment of the integrity of the primary (upper) landfill liner. Completion of this item should be undertaken by October 30, 1983 and must be certified by a New Jersey Licensed Professional Engineer upon completion.
- 10) The owner of the property on which a disposal facility is located shall record, in accordance with State Law, a notation on the deed to the facility property, or some other instrument which is normally examined during title search, that will in perpetuity notify any potential purchaser of the property that:
  - i) The land has been used to manage hazardous waste.
  - b) Post-closure use of property on or in which hazardous waste remains after closure shall never be allowed to disturb the integrity of the final cover, liners, or any other components of any containment system, or the function of the facility's monitoring systems, unless the owner or operator can demonstrate to the Department by petition that the disturbance:
    - i) Is necessary to the proposed use of the property, and will not increase the potential hazard to human health or the environment; or
    - ii) Is necessary to reduce a threat to human health or the environment.
  - c) The survey plat and record of the type, location, and quantity of hazardous waste disposed of within each cell or area of the facility required in paragraph 12 of this approval have been filed with the local zoning authority or the authority with jurisdiction over local land use and the Department.

- II) If at any time the owner or operator or any subsequent owner of land upon which a hazardous waste facility is located removes the waste and waste residues, the liner, if any, and all contaminated underlying and surrounding soil, the owner or operator may add a notation to the deed or instrument indicating the removal of the waste.
  - 12) Within 90 days after closure is completed, the owner or operator of a disposal facility shall submit to the local land authority and to the Department a survey plat indicating the location and dimension of landfill cells or other disposal areas with respect to permanently surveyed benchmarks. This plat shall be prepared and certified by a professional land surveyor. The plat filed with the local land authority shall contain a note, prominently displayed which states the owner's or operator's obligation to restrict disturbance of the site as specified in paragraph 10b of this approval. In addition, the owner or operator shall submit to the Department and to the local land authority a record of the type location, and quantity of hazardous wastes disposed of within each cell or area of the facility.
  - 13) Within 90 days after final closure, NL Industries shall submit to the Bureau of Hazardous Waste Engineering a final topographic map of the landfill. This item must be prepared by a New Jersey Licensed Land Surveyor.
  - 14) Final closure shall not be deemed complete until all New Jersey licensed Professional Engineer's certifications and other submittals required by this approval have been submitted to the Department and approved, and an acceptable post-closure care plan has been approved by the Department. In the interim, NL Industries, Inc., shall continue to comply with all monitoring and reporting requirements of N.J.A.C. 7:26-1 et seq. and the Engineering Design Approval of record for the subject landfill.
- NL Industries, Inc. shall establish within thirty (30) days from the date of this approval, compliance with the liability requirements of N.J.A.C. 7:26-9.13, including the submission to the Department of originally signed duplicates of the insurance policies for sudden and accidental occurrences required by N.J.A.C 7:26-9.13(b) and for non-sudden and accidental occurrences required by N.J.A.C 7:26-9.13(c).
- The New Jersey Licensed Professional Engineer's certifications required by this approval shall be submitted to the Department within seven (b) days after the item being certified has been completed, or within seven (c) days of the date of this approval, if the item has already been completed.

NLI 001 0235

These certifications, along with all submittals to the Department to be made pursuant to conditions 12, 13, 14, and 15 of this approval shall be addressed to:

Frank Coolick, Chief.
Bureau of Hazardous Waste Engineering
Division of Waste Management
New Jersey Department of Environmental Protection
CNO28
Trenton, New Jersey 08625

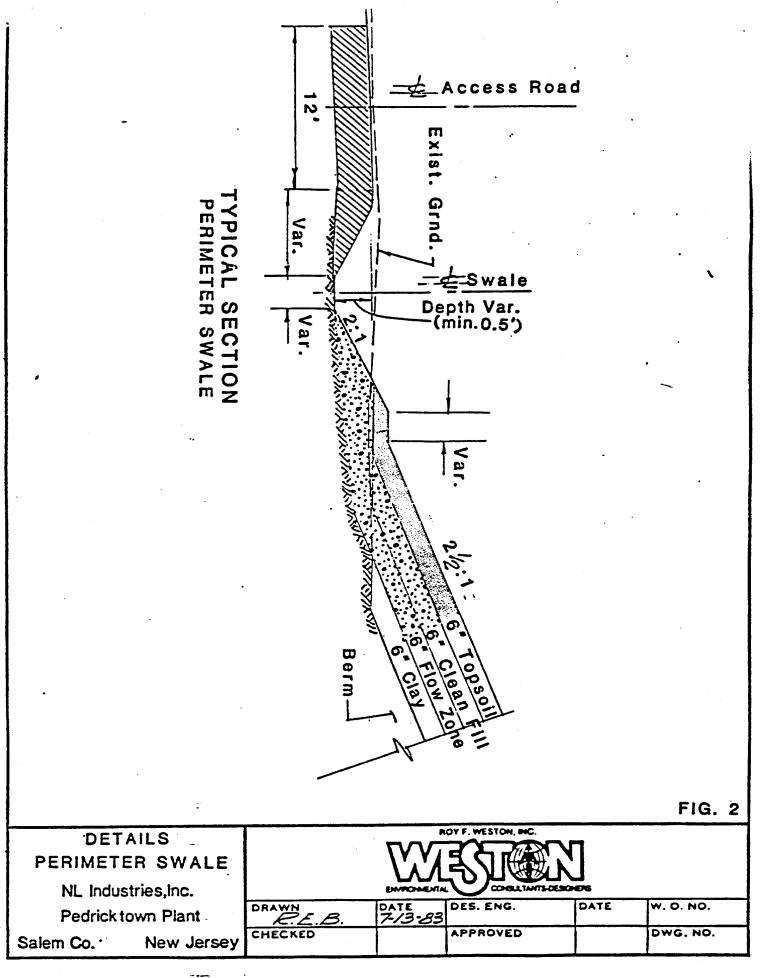
If you have any questions regarding these matters, please call the Bureau of Hazardous Waste Engineering at (609) 292-9880.

Very truly yours,

Edward J Londres Assistant Director

Engineering

EP8/EP10/jb





WESTON WAY WEST CHESTER, PA. 19380 PHONE: {215} 692-3030 TELEX: 83-5348

13 December 1983

Mr. Stephen W. Holt N.L. Industries, Inc. Penns Grove-Pedricktown Road Pedricktwon, NJ 08067

W.O. #0793-08-04

Reference: Landfill Closure Certification ·

N.L. Industries, Inc. Pedricktown, New Jersey Facility No. 1706C EPA I.D.#NJD061843249

Dear Steve:

This is to certify that the closure of the landfill referenced above has been completed by Haas Construction Inc. (prime contractor) in accordance with the following closure plan documents:

- 1. Closure/Post-Closure Plan dated 12/13/82, submitted 12/15/82, prepared by WESTON.
- 2. Addendum report to Closure/Post-Closure Plan dated 3/22/83, submitted 3/24/83, prepared by WESTON in response to NJDEP comments on 12/13/82 Closure/Post-Closure Plan.
- Design clarification letter report dated 8/11/83, submitted 8/16/83, prepared by WESTON. and
- 4. Engineering Design Approval (NJDEP document) dated 4/5/78.

In accordance with the October 27, 1983 meeting between N.L. Industries, WESTON and NJDEP, WESTON provides the following certification for the landfill closure work:

1. A clay cap of a minimum thickness of 6 inches with clay having a maximum permeability of lx10-7 centimeters per second has been placed over the entire landfill surface and has been compacted to at least 90% as determined by the ASTM D698 method. The attached (Attachment #1) certified laboratory results



....

Mr. Stephen W. Holt N.L. Industries, Inc.

-2-

13 December 1983

provided by the Contractor (U.F.&S Reference No. 4617) provides the results of the permeability testing of nine clay samples and indicates thickness of the clay cap at these locations.

- 2. A final cover system consisting of a gravelly sand and sand flow/fill zone and a 6 inch top soil layer was installed and the flow zone meets or exceeds the permeability standards set forth in the closure plan documents. The certified analysis (provided by the Contractor, U.F.&S Reference No. 4580) of the cover system materials is included as Attachment #2.
- 3. The final elevation of the landfill does not exceed 45 feet above mean sea level.
- 4. The side slopes of the landfill are within the limits established by the closure plan documents.
- 5. The final gradient of the completed final cover on the top area of the landfill was measured to be a minimum of 1%.
- 6. Drainage structures including retaining walls around the leachate collection sumps and slope drains were installed and completed in accordance with the closure plan documents.
- 7. Seeding of the final cover was performed and completed in accordance with the specifications. The application rates were established by the Contractor and provided in Attachment #3 (U.F.&S Reference No. 4669-A). The slopes were planted with 100 lbs. of annual rye and 25 lbs. of crown vetch per acre. The top was planted with 50 lbs. of perennial rye, 30 lbs. of Kentucky 31 and 30 lbs. of chewing fescue per acre. Vegetative cover was established exclusive of some areas which may require reseeding in the spring under the post closure plan.
- 8. An 8 foot high chain link security fence has been installed surrounding the landfill.
- 9. Attachment #4, Memorandum from R. Buss to M. Corbin dated November 18, 1983 details the procedure for sounding of the liquid levels in the monitoring sumps in accordance with our meeting of October 27, 1983 and

WESTEN

Mr. Stephen W. Holt N.L. Industries, Inc. -3-

13 December 1983

provides the initial certified sounding measurements from the top of the riser pipes to the liquid levels as recorded on November 18, 1983. A sounding measurement of 28.3 feet was recorded in Sump A and a sounding measurement of 29.9 feet recorded in Sump B.

If WESTON can provide additional information on the closure work performed at the Pedricktown Plant Landfill please contact the undersigned.

Very truly yours,

ROY F. WESTON, INC.

Michael H. Corbin, P.E.

Project Manager

James H. Dougherty, P.E.

Vice President

MHC/JHD/snh

Attachments

cc: Mr. Dean Ervin
 Jeffrey Jacobs, Esq.

### UNDERWOOD, FURMAN & SNYDER TESTING LABORATORIES, INC.

3 South Black Horse Pike Mt. Ephraim, N. J. 08059

William R. Underwood, P. E.

William M. Furman, Manager

Soil Borings - Soil Engineering	- Testing - Inspection - Concrete	- Steel - Asphalt - Masonry
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CLIENT:

Haas Construction Company

PROJECT:

N.L. Industries

11.00201

TEST/INSPECTION REQUIRED: Clay Cover Laboratory Tests

LOCATION:

N.L. Industries, Pedricktown, N. J.

DATE Sampled:

8/10/83

UF&S REF. NO.:

4617

### TEST/INSPECTION RESULTS

All tests and inspections performed on the above landfill were satisfactory and met or exceeded project qualifications.

Below are the results of the most recent tests.

Sample #	Permeability (cm/sec)	Depth <u>of Clay</u>	Location
1	$4.68 \times 10^{-8}$	12.0"	90' Upgradient apron fr. A. Headwall
2	$2.67 \times 10^{-8}$	7.0"	40' S. of N. corner Phase A
3	$3.55 \times 10^{-8}$	17፮''	Center gradient edge slope div. 30' fr
4	$2.07 \times 10^{-8}$	14岁"	Gradient Div. on E. edge of side slope
5	$4.02 \times 10^{-8}$	85"	40' E. of W. corner Phase B
6	$2.61 \times 10^{-8}$	14½"	60' Upgradient fr. B. Headwall
7	$3.99 \times 10^{-8}$	8 3/4"	20' Upgradient fr. B sump wall
8	$4.15 \times 10^{-8}$	7ኔ''	20' Upgradient fr. A sump wall
9	$4.70 \times 10^{-8}$	19 3/4"	20' Upgradient fr. B gas monitoring Well #

Proctor Per A.A. - S.H.T.O. (T 180-72)
Permeability Per, "Falling Head Permeability Test" by K.H. Head in, "Manual of Soil Laboratory Testing, Volume 2. November 1981"

### LABORATORY TEST RESULTS

Proctor Moisture Density Relationship

Test #1 114.6 PLF

Test #2 116.0 PLF

Respectfully submitted,

William R. Underwood, P.E.

### UNDERWOOD, FURMAN & SNYDER TESTING LABORATORIES, INC.

3 South Black Horse Pike Mt. Ephraim, N. J. 08059

William R. Underwood, P. E.

William M. Furman, Manager

Soil Borings - Soil Engineering - Testing - Inspection - Concrete - Steel - Asphalt - Masonry

CLIENT:

Haas Construction Company

PROJECT:

N.L. Industries

REQUIREMENT:

Fill Cover Analysis, Woolwich Sand & Gravel

DATE:

July 26, 1983

UF&S REF. NO.

4580

### LABORATORY ANALYSIS

### SIEVE ANALYSIS

PERCENT PASSING					
SIEVE SIZE	#1 Sand SAMPLE	#2Grave1 SAMPLE	#3 Top Soil SAMPLE		
3/4"	100	90.0	100		
1"	100	100	100		
#4	100	62.0	99.3		
<b>#</b> 16	97.8	49.3	92.7.		
	29.6	9.7	42.6		
<i>‡</i> 100	9.3	3.0	25.0		
<b>#</b> 200	2.9	1.2	10.0		
Permeabili	ity 2.5 X 10 <sup>-2</sup>	7.5 X 10 <sup>-2</sup>	1.8 x 10 <sup>-2</sup>		

UNDERWOOD, FURMAN & SNYDER TESTING LABORATORIES, INC.

William R. Underwood, P.E.

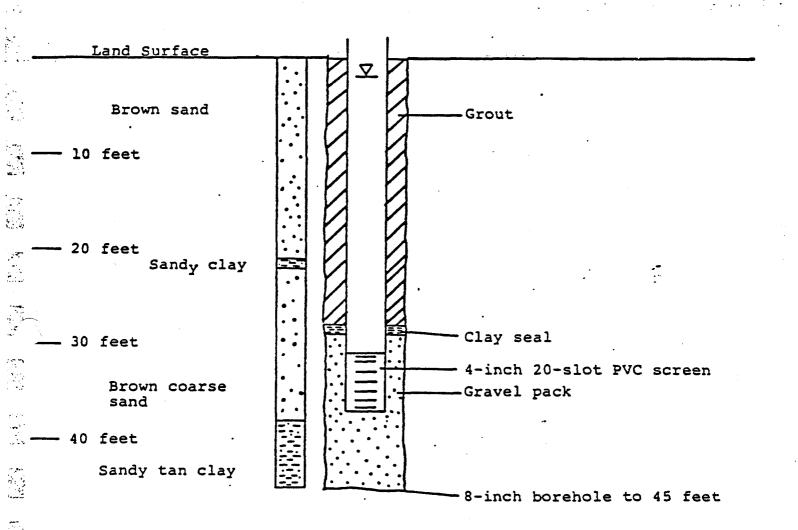
CC to Mr. M. Corbin, Weston Way

West Chester, Pa.

## A.C. OCHULTES & SONS. INC.

	- 3	WELL LOG	FEET -	NAME OF OWNER
•		Top Soil	FROM GROUND SURFACE	National Lead
· · ·		100 3011	0 70 4: -	
. •		Brown Sand-Coarse to	1 - 26	Job = 18411
		Medium Clay	26 - 27	Location Pedricktown, N
,		Coarse Brown Sand	27 - 38	Weil No. B-R
		Clayish	• 38 - 45	His. PumpedAir Devel. 2
	:		•	Capacity G.P.M. 30
	:			Static Level 4
				Pumping Level
				Specific Connecty
				Diameter of Well 4"
				Depth of Well (ground) 37 ° 0 "
				Length of Cosing 33 f
				Distance to Top of Packer (gr.)
	-CASING-			Type Screen PVC
			-	Size of Screen 4"
				Length of Screen 6 ° 0 "
				Top Screen Firting PVC Socke
6 ° RAINER				Bottom Screen Fitting Cap
1 2 2	===[	•		Blenis
		٠.		Slot Size . 018
			. •	Drilling Machine No. 6B
				DrillerC. Sacco
	:			Gravel #1
				Bogs of Coment 10
				Date Well Completed 1/1/80
•	· . l	÷ -	•	

N/L INDUSTRIES, INC.
Pedricktown, New Jersey
Completion Diagram for Well BR



Well completed April 1 1980 <sup>-</sup>

▼ SWL = 1.9 feet below grade, April 3 1980

Leggette, Brashears & Graham, Inc.

April 1980

WELL LOG
LEGGETTE, BRASHEARS & GRAHAM, INC.
CONSULTING GROUND-WATER GEOLOGISTS
72 DANBURY ROAD

- WILTON, C			PAGE 1- OF 1 PAGES
эсасыяток.	THICK. HENG USETI	DEPTH WEETI	Owner NL Industries. Inc.
Sand, gray, fine to medium	· 4	4	Pedricktown, NJ
Clay, gray	4	. 8	CR2
Clay, red and white, hard, impermeable	e 15	23	SATE COMPLETES March 29, 1980
Sand, brown, fine	10	33	Danuma A. C. Schultes & Sons
Clay, light gray-white, silty	4	37	Describe Water rotary
Sand, very fine, with silt & clay	5	42	SAMPLING Ditch
Clay, white, silty	3	45	Samples Sandy Strausberg
			REFERENCE grade
			ELEVATION OF R. P.I.
	·		4-inch
			Slotted PVC
			Bun. 4-inch stor No. 20
		. •	Setting 25-31
			PUMPING TESTS DATE:
			DURATION
			STATIC WATER 8.0 feet*
			PUMPING WATER
			15 gpm
· · · · · · · · · · · · · · · · · · ·			
			* below top of 4-inch
			PVC casing
d	·		
L.			1
5			NLI 001 0245
	1		101 001 0240

## A.c.S CHULTES & SONS, INC.

GROUND		2" SINGLE	CASE WELL	
	-	WELL LOG	FEET FROM GROUND SURFACE	NAME OF OWNER
		Gray Sand-Med. to Fine	0 ТО 4	National Lead
		Gray Clay	4 - 8	Job = 18411
		Red & White Clay	8 - 23	Location Pedricktown,
		Fine Brown Sand	23 ÷ 33	Well No. C2R
		White Clay	33 - 37	Hrs. Pumped Air Devel.3 .
		Fine Brown Sand	37 - 42	Copocity G.P.M. 20
	-	White Clay	42 - 45	Static Level
				Pumping Level
į.				Specific Capacity
. I .				Diameter of Well 4"
, DEI				Depth of Well (ground) 31 0 "
- TOTAL				Length of Casing 27'0"
•				Distance to Tap of Packer (gr.)
	CASING-			Type Screen PVC
			•	Size of Screen 4"
			·	Length of Screen 6"0"
			·	Top Screen Fitting PVC Socke
6" STRAINER-				Bottom Screen Fitting Cap
ATS-				Blank
		٠		Slot Size . 018
				Drilling Machine No. 6B
•	•			Duller C. Sacco
				Gravel #1
	_	•		Bogs of Coment 10
				Date Well Completed 3/29/80
	-			
	•.	-	<u> </u>	
	•,	P.	ary Table sancer. T store ora	and laund

### CONSULTING GROUND-WATER GEOLOGISTS

72 DANBURY ROAD

WILTON, C			PAGE 1 OF 1 PAGES
DESCRIPTION	THUCK- MENE WEETT	DEPTH PRETI	OWNER N/L Industries, Inc.
Sand, medium, light brown, fill	1.5	1.5	Locamon, Pedricktown, NJ
Sand, fine, brown-black, silty, with			wan well (Replacement)*
wood	3.0	4.5	DATE December 14, 1979
Sand, fine to medium, brown-black,			COMPANY Craig Testing Labs,
with organic material	7 5	12	Maries Mud rotary (bentonit
Sand, fine, silty	_1_	13	Markon Ditch and 1 split sp
Silt, clayev, brown-black, with gravel	2 .	15	EXAMINED SY. Lee Criibman
Sand, fine to medium, brown-black.			POINT Land surface
with silty clay lumps	<u>5</u>	20	ELEVATION OF R. P.
Sand, medium, tan-black, some fine	_	•	CASING
sand and silt	5	25	Scheme Slotted PVC
Sand, medium to coarse, tan, some fine gravel	ų	29	вым. 4 inch ster но 20 4-32 feet
Clay, white, with sand, medium to coars		7.5	Punming Tests
and gravel, fine	3	32	DURATION
Clav. red. sandy	3.5	35.5	TATHE WATER 3.6feet (12/17/79
374.			PUMPING WATER
		<u>.</u> .	that the thing that the thing the thing the things the
			during devel-
			* Located ±25 feet east- southeast of original
:			well #1
			*
		1 2 2	
ri-tyrriferi i i i i i i i i i i i i i i i i i i			

## WELL LOC

LEGGETTE, BRASHEARS & GRAHAM, INC.

### CONSULTING GROUND-WATER GEOLOGISTS

72 DANBURY ROAD PAGE 1 DF 1 PAGES WILTON, CT. 06897 THIOT. NL Industries, Inc. PERM LOCATION: Pedricktown, NJ Sand, brown, fine to medium, some WELL NO. 2R2 7 7 coarse Compare October 15, 1980 Sand, fine to medium, with much COMPANY A. C. Schultes & Sons, coarse to very coarse sand and METHOD Water rotary 18 some fine gravel 11 METHOD: ditch Gravel, brown, fine, subrounded, Samples: Sandy Strausberg some medium gravel and fine to REPERENGE POINT. Land surface verv coarse sand ... 3 21 er R. P. ± 7.2 feet MSL Clay, red and white, silty 25 SCHEEN-PVC DIAM: 4-inch SLOT No. 20 servine: 13-20 feet Pumpage TESTA October 16, 1980 4 hours DURATION: ETATIC WATER 5.52 feet below grade Pumme water 7.79 feet below grade YIELDI 6 gpm REMARKS Hole to 25 feet Gravel pack (Morie #1) 7-25 feet Bentonite 7 feet to surface Casing ID = 4 inches NLI 001 0248

# EGGETTE, BRASHEARS & GRAHAM, INC. CONSULTING GROUND-WATER GEOLOGISTS

WILTON, C	T. 06	897 <u>.</u> .	PAGEOFPAGES
DESCRIPTION .	THE CEL	DEPTH PERTI	Owner N/L Industries, Inc.
Sand, fine, light brown, silty	3	3	Lecanou Pedricktown, NJ
Sand, fine to medium, light brown,			wmr wo.3 (Replacement)*
silty	ц	7	Date December 17, 1979
Sand, fine to medium, with some fine			Company Craig Testing Labs,
gravel and silt -	42	113	DRILLING Mud rotary (bentoni
Sand, fine to medium, light brown,			SAMPLING Ditch
with some fine—to medium gravel	4	15½ .	EXAMINED TO Lee Grubman
Sand, fine to medium, brown, with.			POINT Land surface
white clay lenses	5	203	ELEVATION OF R. P.J.
Clay, tan, fine-sandy	23	23	CASIMO
Sand, fine to medium, brown, with gra-			Slotted PVC
vel, fine and little silt and clay	4	27	Dun, 4 inch stor Mc 20
Sand, medium to coarse, tan, some fine gravel	1	28	SETTING 4-33 feet
Sand, medium to_coarse, reddish-tan,	-	20	DATE
some fine gravel, with white clay			STATIC WATER ±4 feet after
lenses	23/2	31½	drillin
Clay, red-brown, with sandy lenses	13	33	±4 gpm during develo
			ment
(A sample of red clay was taken from			* Located ±15 feet south
the drill bit)			west of original well
		•	
	1		

WELL LOG Leggette, Brashears & Graham, Inc.

### CONSULTING GROUND-WATER GEOLOGISTS

72 DANBURY ROAD WILTON CT D6897

	WILLION, C	1. 06	0071	PAGE PAGES
•	DESCRIPTION :	THIOK.	DEFTH VEETS	NL Industries, Inc.
• • •	Sand, brown, fine to medium, little			••
-	coarse sand	15	15	Location, Pedricktown, NJ
	Sand, brown, fine to coarse, silty	6	. 21	WELL No.: 4R
	Sand, silty, grey	1	22	DATE 7-21-80
	Clay, white and tan, some red, sandy	4	26	DRILLIMO A.C. Schultes & Sons,
-	Clay, red, with occasional rounded			Inc.  DRILLIMG Water rotary  METHOD
	white small cobbles	3	29	SAMPUNG ditch
				Sammes: Lee Grubman
	•			REFERENCE land surface
:				ELEVATION OF R. P.
- <del>-</del> _;	<u> </u>	,		GASING. 4-inch PVC
	•			SCREEND PVC
· · · .				Plant 4-inch stor No 20
s 3 <sup>-</sup>				SETTIME: 9-21 feet below grade
** <b>4</b>				PUMPING TESTIN
				Duration
				STATIC WATER 8.9 feet below gra
;				Pummme Waterlo.6 feet below
<u>.</u>				
-, -				7.9 cpm
				PVC stickup=3.0 feet
				sand pack 7-25 feet bentonite 4-7 feet
				grout to surface
				•
			· · · · · · · · · · · · · · · · · · ·	·

### UNDERWOOD, FURMAN & SNYDER TESTING LABORATORIES, INC.

### 3 South Black Horse Pike Mt. Ephraim, N. J. 08059

William R. Underwood, P. E.

William M. Furnan, !

Soil Borings - Soil Engineering - Testing - Inspection - Concrete - Steel - Asphalt - Masonry

CLIENT:

Haas Construction Company

PROJECT:

N.L. Industries

REQUIREMENT:

Fill Cover Analysis, Woolwich Sand & Gravel

DATE:

July 26, 1983

UF&S REF. NO. 4580

### LABORATORY ANALYSIS

### SIEVE ANALYSIS

PERCENT PASSING #1 Sand #2Gravel #3 Top Soil SIEVE SAMPLE SAMPLE SIZE SAMPLE 3/4" 100 90.0 100 100 100 100 100 62.0 99.3 **\$**4 92.7 97.8 49.3 #16 9.7 42.6 29,6 **#50** 3.0 9.3 25.0 #100 1.2 2.9 10.0 #20D.  $1.8 \times 10^{-2}$  $7.5 \times 10^{-2}$ Permeability 2.5 X 10<sup>-2</sup>

UNDERWOOD, FURMAN & SNYDER TESTING LABORATORIES, INC.

WITH RUM

William R. Underwood, P.E. CC to Mr. M. Corbin, Weston Way West Chester, Pa.

# VELL LOG LEGGETTE, BRASHEARS & GRAHAM, INC.

### CONSULTING GROUND-WATER GEOLOGISTS

72 DANBURY ROAD

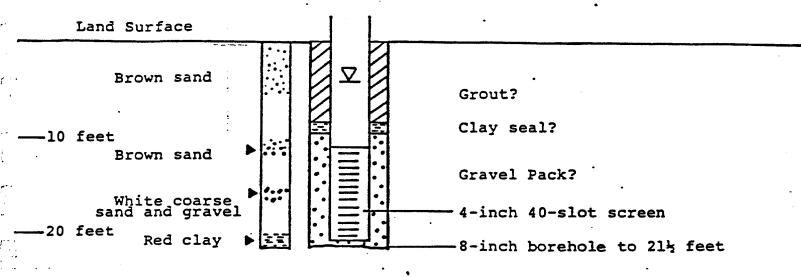
WILLON, C	1. UD	031	PAGE OFPAGES
D ED CRAPTION	THICK- NESS UPERTI	PEPTH PEETI	OWNER NL Industries, Inc.
Sand, dark brown, fine to coarse,	0	3.	Pedricktown, NJ
with silt and decayed plant		•	WELL No. 5R
material			DATE 7-22-80
Sand, tan, fine to medium	3	4	DRILLING A.C. Schultes & Sons
Sand, grey to white, medium, some			Inc  DRILLING Water rotary  METHOD:
coarse sand, little fine gravel	4	20	SAMPLING ditch
Sand, white, medium, and white			SAMPLES: Lee Grubman
clay bits, some coarse sand and			REFERENCE Land surface
fine gravel	20	22	ELEVATION OF R. P.I.
Sand, grey to white, -medium	22	29	Gasine: 4-inch PVC
Sand, grey to white, fine with			SCREENL PVC
white clav	29	32	4-inch 20
Clav, tan to white	32	34	7-16 feet
Clav, tan grading-to solid red			Pumming TERTIN DATE:
clav with occasional rounded			DURATION
white small cobbles	34	35₺	STATIC WATER 3.5 feet below gra
	133		
			PUMPHE WATER 7.9 feet below gra
			7.9 gpm
			PVC stickup=25 feet
			Sand pack 5-28 feet Bentonite 2½-5 feet
		7	Bentonite 23-3 leet
			-
•			-
	•	,	NLI 001 0252

# LEGGETTE, BRASHEARS & GRAHAM CONSULTING GROUND WATER GEOLOGISTS 55 WEST STATE STREET: WESTPORT, CONNECTICUT

	<del></del>	<del></del>	
DESCRIPTION	THICK- NEBS (FEST)	*DEFTH **	oN.L. Industries
Sand, brown, fine to medium;		.: .	Pedricktown, N. J.
5% dark minerals	5.0	5.0	LOCATION
. CORE 5.0' to 6.5' ±1.0' recovery	X		WELL NO.
Sand, as above	1.5 <u>b</u>	6.5	DATE October 8, 1976
CORE 10.0' to 11.5' ±1.5' recover	n y		Danking Craig Testing Labs.
Sand; brown, fine to medium;			DRILLING 8-inch auger
±5% dark minerals (upper 1.0')		·	SAMPLING Split spoon
Sand, grey, fine to medium with			SAMPLES: M. R. Burke
%-inch chert nodules, fine			REFERENCE Land surface
quartz gravel (lower 0.5')	1.5 <u>b</u>	11.5	ELEVATION ±10 feet above MSL
CORE 15.0'to 16.5'±1.5' recovery			13 feet of 4-inch P.
Sand, white, very coarse; grave	1,		SCREENL P.V.C.
medium, immature; feldspar,			4-inch 0.040-
angular and rounded quartz, som	e		ll ft. to 21 ft.
k-inch quartz granules	1.5 <u>b</u>	16.5	Pumping Testh Date:
CORE 20.0' to 21.5' ±1.5' recover	У		DURATION:
Sand, white, coarse to very			STATIC WATER 6.52 feet below
coarse; gravel, fine; k-inch			OI Casing a/
quartz granules (upper 0.71)			2 gpmc/
Clay, red with white streaks,	·		;
plastic; very few quartz			a/ 2.30 ft. stick up
granules (lower 0.8')	1.5 <sup>b</sup>	21.5	b/ Penetration in feet
			c/ Discharge milky with no sand.
			Water at 3.5 feet below
		-	land surface
			• .
			1

NLI 001 0253

N/L INDUSTRIES, INC.
Pedricktown, New Jersey
Completion Diagram for Well 6



▶ Split spoon sample

Well completed October 8 1976

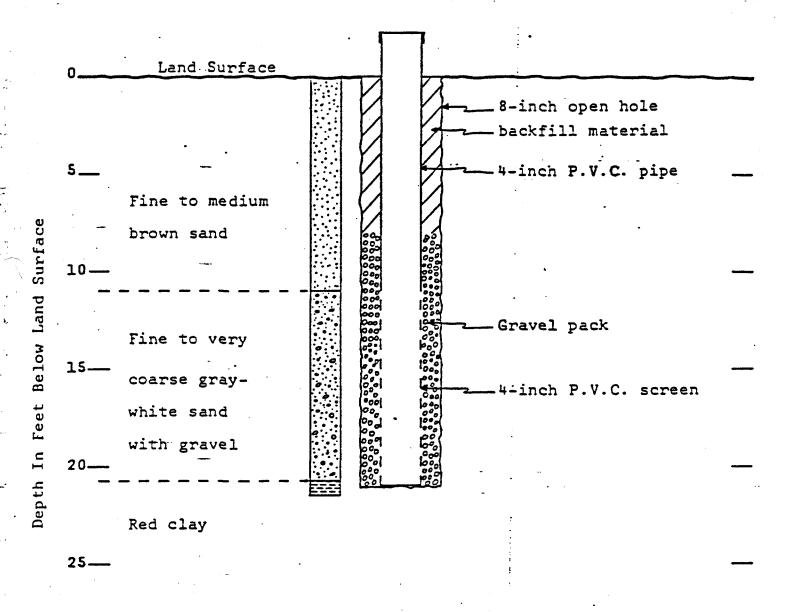
▼ SWL = 4.2 feet below grade, April 3 1980

Leggette, Brashears & Graham, Inc.

April 1980

### N.L. INDUSTRIES Pedricktown, New Jersey

· Completion Diagram for Observation Well 6.



Leggette, Brashears & Graham, Inc. October, 1976

WELL LOG
LEGGETTE, BRASHEARS & GRAHAM, INC.
CONSULTING GROUND-WATER GEOLOGISTS

72 DANBURY ROAD - WILTON, CT. 06897 - -

- WILTON, C	T. 06	897 -	- PAGE OF PAGES
BESCRIPTION	THEOR.	Interva	lowner NL Industries, Inc.
Of reco	core		
To save time the driller was instructe	đ		LOCATION Pedricktown, NJ
to begin split spoon sampling at 10			Wal No. 8R (Replacement)
feet below grade. Upper materials	<u> </u>		DAYE March 19, 1980
based on landfill cut are brown sand,			COMPANY A. C. Schultes & Son
plastic fill, and some clay lenses		0-10	DRILLING Mud Rotary
Sand, fine to medium, brown	.5	10-11.5	SAMPLING Split spoon
Sand, medium, some fine, red and			SAMPLES: EN R. Lamonica
brown; trace clay	.5	15-16.5	REFERENCE Grade
Sand, very fine to coarse, brown;			ELEVATION OF R. P.L.
trace clay	.5		easume 4-inch PVC Plastic
Sand, very coarse, and clay, white	. 2	20-21.5	SCREEN Slotted PVC Plastic
Sand, fine, white	.8	25-26.5	Blam 4-inch ster No. 020 in
Sand, fine-medium, brown	.6		Barnes 101-108 feet
Sand, fine-medium, white, with streaks			Pumme Testo Date
of clay, white; few h inch gravel	.25	30-31.5	DURATION
Driller reported formation change to			STATIC WATER 20.35 feet below
clav		32	PUMPING WATER
Clav. grav. some vellow streaks, very			l gpm
stiff	1.0	35-36.5	
Clay, predominantly red and brown,			Remarks
some gray, mixed with sand, coarse	1.0	10-41.5	
Sand, fine, clayey, light gray, yellow			
streaks	1.0	15-46.5	
Clay, with fine sand, light gray,			
vellow streaks	1.0	50-51.5	
(Continued)			
	Į.	1	NLI 001 0256

WELLLOG
LEGGETTE, BRASHEARS & GRAHAM, INC.
CONSULTING GROUND-WATER GEOLOGISTS
72 DANBURY ROAD

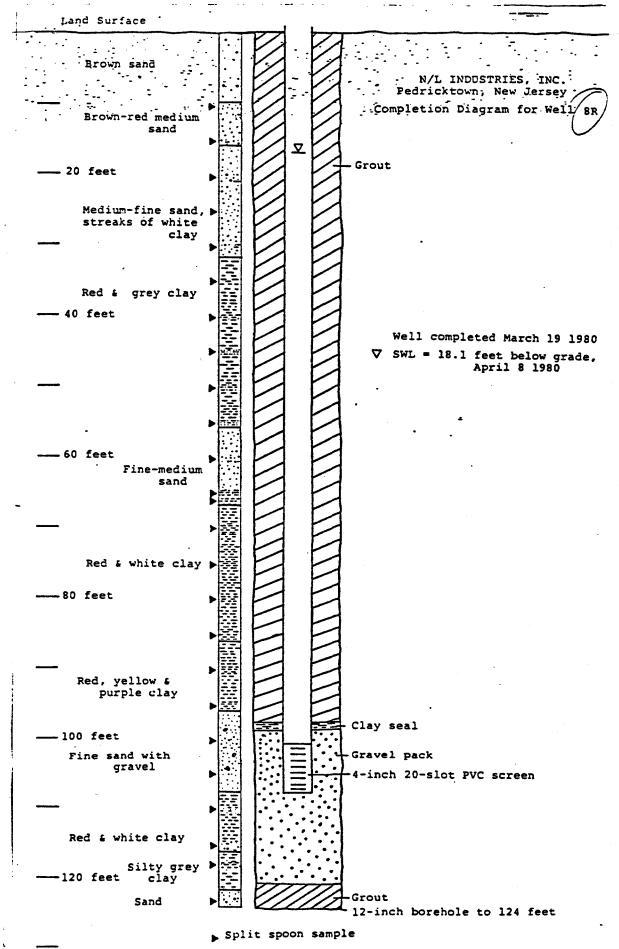
_	WILTON, C	T. 00	897	PAGE 2 OF 3 PAGES
Ų	B to con-viola	THIGH.	PEPTH WESTI	OWNER NL Industries Inc.
_		<u> </u>		
	Sand, fine, clayey, light gray	عبدا	55-56-5	Locum-Pedricktown, NJ
· ,	Sand, fine to medium, light gray;	<u> </u>		war No. 8R (Replacement)
	trace clay	.25	60-61.5	DATE COMPLETED
· · · -	Clay, plastic, white; trace sand.			DRILLING COMPANY
. —	medium	1.10	64_5-66	Den Line Method
_	Clay, plastic, red, some white	1.0	66-67.5	Sampling Method
·	Clay, stiff, red, some light gray	1.0	70-71.5	SAMPLES
	Clay, stiff, mottled red and light	<u> </u>		REPERENCE
·	gray	1.5	75-76.5	ELEVATION of R. P.J.
'. ——	Clay, stiff, mottled red and light		·	CASIMS
•	gray	1.0	80-81.5	Screen.
<u> </u>	Clay, stiff, mottled red and purple.			DIAMSLOT NO
· 	some light gray	1.0	84.5-86	\$ STTING!
	Clav. very stiff. mottled. mostly			Pusping Tests DATE
: 	light gray with red, purple and			DURATION
<i>:</i> , —	vellow	1.0	90-91	STATIC WATER
	Driller reported possible change of		·	PUMPING WATER
• <del></del>	formation .		92	CIVILL
	Clay, very stiff, mottled, mostly		÷	THE
	light gray with red, purple and		:	REMARKS
_	yellow	.2	ţ	
· . ·	Sand, fine, clayey, bright yellow	.4	95-96	
· _	Sand, fine to medium, light gray and		·	
	vellow	5_	100-101	
<u> </u>	SAMPLES FROM HERE ON ARE SHELBY TUBE			
-	Sand, fine to coarse, yellow, trace			NLI 001 0257
		_	-	

WELL LOG
LEGGETTE, BRASHEARS & GRAHAM, INC.
CONSULTING GROUND-WATER GEOLOGISTS
72 DANBURY ROAD
411 TON CT 05897

WILTON, C	. Ť. O	897	PAGE 3 OF 3 PAGES
D ED CAMPTION	YHICK. HENG IP SATTI	DEPTH WESTI	OWNER NL Industries; Inc.
	-	104.5-	Lecanon Pedricktown, NJ
clay: chips of sandstone, brown Driller reported change to harder	1.5	106	WELL NO. 8R(Replacement)
material	1	108	DATE COMPLETED
Sand, fine, silt, yellow; lenses of		100	Darline Company
sandy clay, white, and clay, white			Dantima Mathema
and red	.8	110- 111.5	BAMPLING METHOD
Sand, fine to medium, light gray;			BANFLED: EXAMPLED:
some sand, fine to very coarse.			REFERENCE POHT
light gray; few gravel; traces of	<u> </u>		ELEVATION OF R. P.I.
white clay throughout		114.5- 116	CASIMO
Driller reported change to harder	ļ		SCATOL- TYPE
material		116 118-	DIAM. SLOT NO.
Silty clay, light gray, very hard	1.0		SETTING
DRILLED SAME AS ABOVE TO 122 FEET	<b>_</b>		PUMPING TESTS DATE
BELOW GRADE; TOOLS THEN DROPPED VERY			BURATION
OUICKLY TO 124 FEET STOPPED DRILLING			STATIC WATER
Sand, fine to coarse, yellow, with		124-	PUMPING WATER
clay	.5	124.5	YIRD
ADDED 20 GALLONS (300 pounds) OF			REMARKS:
CEMENT TO THE BOTTOM OF THE 12-INCH	-		
HOLE TO CLOSE OFF LOWER SAND ZONE			
	-		
	1		
	1	<del></del>	NLI 001 0258

## A.C. OCHULTES & SONS, INC.

••		-	SINGLE	CASE WELL	
			Brown Sand	FROM GROUND SURFACE	NAME OF OWNER
-	<del>-</del>	-	Brown & Red Med. Sand	10 - 16	National Lead
		-	Med Fine Sand and	16 - 32	Lecesien Pedricktown, A
		1	streaks of white clay		well No. 8R
		_	Red & Gray Clay	. 32 - 56	Hrs. Pumped 8 hrs.
			Fine to Med. Sand	56 - 67	Capacity G.P.M. 9 gal.
		•	Red & White Clay	67 - 86	Static Level 18'
		. <u>.</u>	Red, Yellow & Purple	86 - 96	Pumping Level 93'
108-0 TOTAL DEPTH - FT	;	•	Clay Fine Sand & Some Sand-	96 - 108	Specific Copocity
		_	Red & White Clay	108 - 116	Diameter of Well 4"
			Silty Gray Clay	116 - 122	Depth of Well (ground): 108-0
	•		Sand . —	122 - 124	Length of Casing 101-8
		4-			Distance to Top of Packer (gr.)
		CASING-			Type Screen PVC
	. 1	— —			Size of Screen 4"
					Length of Screen 7'0"
	. 5			•	Top Screen Fitting PVC Socke
	7 ' 0"				Battom Screen Fitting Cap
	5			•	Blank
					Slet Size . 018
	+	,			Drilling Machine Na. 6B
				•	Driller C. Sacco
••					Grevel #1
•.	•				Bags of Cement 100
•				•	Date Well Campleted 3/20/80
•	•				
		. [			<u> </u>



Loudcite, Brashears & Graham, Inc. April 1980

### WELL LUG

LEGGETTE, BRASHEARS & GRAHAM, INC.

CONSULTING GROUND-WATER GEOLOGISTS

72 DANBURY BOAD

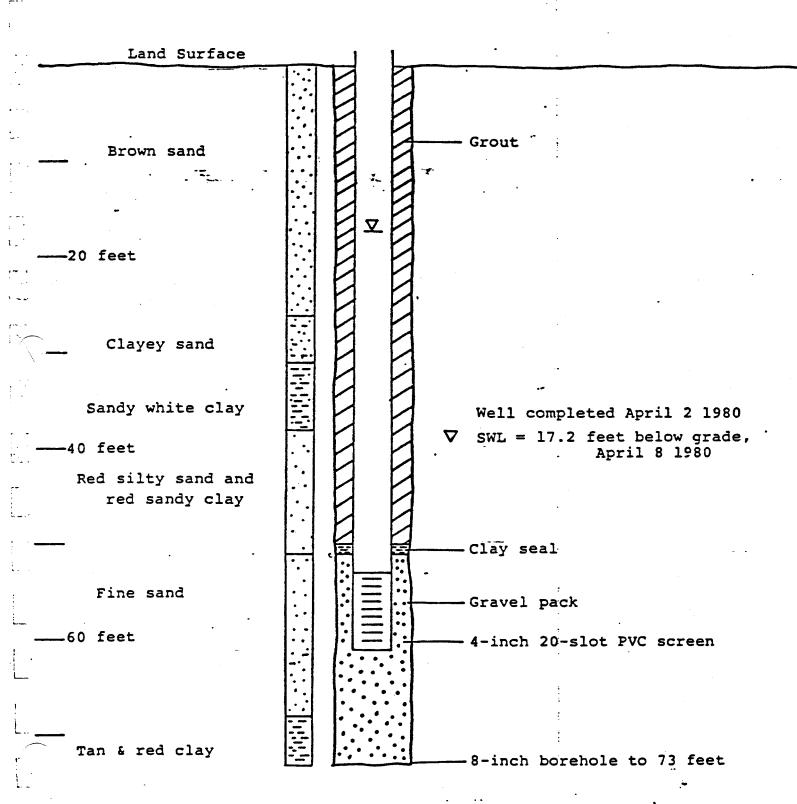
- WILTON, CT. 06897 -OWNER NL Industries, Inc. Sand, brown 26 26 LOCATION PEDTICKTOWN NJ Sand, clayey 5 31 9R2 Clay, white, sandy 7 DATE April 2, 1980 38 Sand, red, silty, and red sandy clay COMPAND A. C. Schultes & Sons 13 51 Sand, with silt and clay Married Water rotary 3 54 . بر د Sand, fine 14 68 Clay, tan and red Lee Grubman 73 REFERENCE grade Casme 4-inch Seem-Slotted PVC DIAM 4-inch SLOT NO. 20 53-61 PUMPING TESTS DURATION. STATIC WATER 17.1 feet\* PUMPING WATER 10 gpm REMARKS \*below top of 4-inch PVC casing. NLI 001 0261

### A.C.OCHULTES & SONS INC.

<b>:</b>	.GROUND -	- · ·	2'0" SINGLE	CASE WELL	
	-		Brown Sand	FEET FROM GROUND SURFACE O TO 26	National Lead
		7	Clayish Sand	26 - 31	Job = 18411
			White Clay	31 - 38	Location Pedricktown, N
			Red Silty Sand & Clay	38 - 51	Well No. 92R
	:	<u>.</u>	Clayish Silty Sand	. <b>51 - 54</b> ·	Hrs. Pumped 8 hrs. +
	•		Fine Sand :	54 - 68	Copecity G.P.M. 9
			Red & White Clay	68 - 73	Static Level 16"
7.				•	Pumping Level 45
					Specific Copacity
0"		-			Diometer of Well 4"
61' OTAL					Depth of Well (ground) 61 0 "
Ī	•				Length of Casing 55'
		CASING-	- :		Distance to Top of Pocker (gr.)
		4"			Type Screen PVC Size of Screen 4"
					Length of Screen 8'0"
					Top Screen Fitting PVC 50c)
İ	8 ' 0" Strainer-				Bottom Screen Fitting Cap
	8 '8 -				Blank
					Slot Size . 018
		]===[			Drilling Machine No. 6B
			-		Driller C. Sacco
			-		Grave! #1
					Bags of Coment 25
				•	Date Well Completed 1/2/80
					<u> </u>
		. 1			

Retary Table approx. 3 above ground level

N/L INDUSTRIES, INC.
Pedricktown, New Jersey
Completion Diagram for Well 9R2



Leggette, Brashears & Graham, Inc.

April 1980

### Geologic Logs

		epth	<b></b>	
Description		t below surface)	Thickness (feet)	
	<del> </del>			
Well 10				
Sand, brown, fine; with a trace of silt	0	- 4	4	
Sand, gray, fine	4	- 14	- 10	
Sand, gray, fine to coarse	14	- 20	6	
Sand, gray-white, fine to coarse; with silt, clay and occasional lenses				
of clayey sand	20	- 28	8.0	
Clay, red-pink, white, mottled; with				
silt	28	- 33	5	
Silt, white; with some clay and very			_	
fine sand	33	- 41	8	
Sand, gray-white, very fine; with silt and occasional lenses of silty				
clay	41	- 49	8	
Sand, red-brown, fine to medium; with occasional gray-white lenses of			·	
sandy silt	49	- 63	14	
Clay, red-pink, white, mottled; with				
silt	63	- 66	3	
Sand, red-brown, fine to coarse	66 <sup>*</sup>	- 73.5	7.5	
Clay, red, brown and white, mottled;				
with silt	73.5	- 79	5.5	
Sand, red-brown, fine to medium; with lenses of silt, clay and gray				
sand	79	- 82	3	
	• •	~-	•	

ORETRENCH AMERICAN WELL REPORT	FIIR	Rig No.: 0 4-1 5
ontractor:		Job No.: 1-4235
lob Address: M.L. = PEPRICK TOWN	. <u>کرائزر</u> .	Date: 10-14-83
ob Phone: Comments:		
Branch No.:		
No. of wells installed today:	<del></del>	Depth of hole: 60 6
Dis of borehole: 12"		Length of well: 25'
Dia, of well: 4"		Length of screen: 20
Type, dia., slot of screen		Well yield:
Travel Hours Stand by Hours		Revert used:
fan Hours Regular: 🔀 8 Drill Hours Reg.: 🗜	28	Total footage drilled today: 506
Overtime: 4 O.T.:	4	
elays-Explain: WAITING FOR DEPTH	Dela	y Hours:
TO SET WELL		
		••
Miles	- 1 1	Fuel Supplied By:(check one)
Mileage	,	☑-Drittler
State w/		☐ Contractor
WELL	LOG	
Depth Formation	Depth	Formation
0-1' T.P : 501L		
1'-6' FINE- MED. BAN, GAND	•	
6-12' MED-FIND BON. GNP		
17-18 YOARSE-MED, BON. AND	•	
17-18 YOARSE-MED. BON. GND	•	
TRACE OF GOAVEL	•	
TRACE OF GRAVEL  18-26 FINE-MED. BAN, SAND		
TRACE OF GRAVEL  18-26 FINE - MED. BAU. SAND  TRACE OF GRAVEL		
TRACE OF GRAVEL  18-26 FINE-MED. BAN, SAND  TRACE OF GRAVEL  2678 MED-COARSE SAND		
TRACE OF GRAVEL  18-26 FINE-MED. BAU. SAND  TRACE OF GRAVEL  2678 MED-COARSE SAND  37:44' MED-COARSE SAND		
TRACE OF GOAVEL  18-26 FINE-MED. BOW. SAND  TRACE OF GOAVEL  2678 MED-COARSE SAND  35' 44' MED-COARSE SAND  TRACE-FINE GOAVEL		
TRACE OF GRAVEL  18-26 FINE-MED. BAU. SAND  TRACE OF GRAVEL  2678 MED-COARSE SAND  38: 44' MED-COARSE SAND  TRACE-FINE GRAVEL  44' 48 TOARSE-MED BAN. MAD		
TRACE OF GRAVEL  18'- 2' FINE - MED. BAN, SAND  TRACE OF GRAVEL  2' JA MED-COARSE SAND  3F' 44' MED-COARSE SAND  TRACE-FINE GRAVEL  44' 48' COARSE-MED BAN, MAD  48'- 36' MED-COARSE TAN SAND		
TRACE OF GRAVEL  18'- 26 FINE - MED. BAN. SAND  TRACE OF GRAVEL  2678 MED-COARSE SAND  38' 44' MED-COARSE SAND  TRACE-FINE GRAVEL  44' 48' COARSE-MED BAN. MND  48'- 36' MED-C-ARSE TAN SAND  56' 66' MED-C-ARSE TAN SAND		
TRACE OF GRAVEL  18'- 26 FINE - MED. BRU, SAND  TRACE OF GRAVEL  2678 MED-COARSE SAND  3F' 44' MED-COARSE SAND  TRACE-FINE GRAVEL  44' 48' COARSE-MED BRU, MND  48'- 36' MED-COARSE TAN SAND  56- 666 MED-COARSE TAN SAND  KLACE FINE GRAVE	•	
TRACE OF GRAVEL  18'- 2' & FINE - MED. BAN. SAND  TRACE OF GRAVEL  2' & 7' & MED-COARSE SAND  38' 44' MED-COARSE SAND  TRACE-FINE GRAVEL  44' 48' FOARSE-MED BAN. MND  48'- 36' MED-C-ARSE TAN SAND  56: 66 MED-C-ARSE TAN SAND	•	
TRACE OF GRAVEL  18'- 26 FINE - MED. BIN, SAND  TRACE OF GRAVEL  26 78 MED-COARSE SAND  38' 44' MED-COARSE SAND  TRACE-FINE GRAVEL  44' 48' POARSE-MED BIN, MND  48'- 56' MED-COARSE TAN SAND  56- 66' MED-COARSE TAN SAND  TRACE-FINE GRAVE	•	

NLI

Verified Contractor

# EXHIBIT C OBSERVATION WELL LOGS

Geraghty & Miller, Inc.

### Installation of Observation Wells

During the field investigation, 28 observation wells were screened in the water-table aquifer, and two deep wells were screened in the first artesian aquifer to determine hydrogeologic conditions. Figure 1 is a map of the plant site showing well locations and lines of section. Figure 2 shows the construction details of a typical well cluster screened in the upper and lower zone of the water-table aquifer. Tables 1 and 2 provide construction details of the monitoring and observation wells. Geologic logs of the observation wells are included in Appendix A.

Testwell Craig Test Boring Company of Mays Landing, New Jersey, installed the wells with a power auger under Geraghty & Miller, Inc.'s direction. At each location, a 12-inch diameter hole was drilled to the required depth with split spoon samples collected at 5-foot intervals in wells completed in the lower water table zone. Shelby tube samples were collected from the confining clay layer, separating the water-table aquifer and the first artesian aquifer, at well locations T4 and 10. The results of laboratory permeability determinations of these samples are provided in Appendix C. The elevation of each well (top of PVC casing) was surveyed and converted to mean sea level by Albert A. Fralinger, of Bridgeton, New Jersey.

### Excerpted from:

Geraghty & Miller, Inc., "Hydrogeologic Study and Design of Groundwater Abatement System at NL Industries, Inc. Pedricktown, New Jersey Plant Site," May 1983.

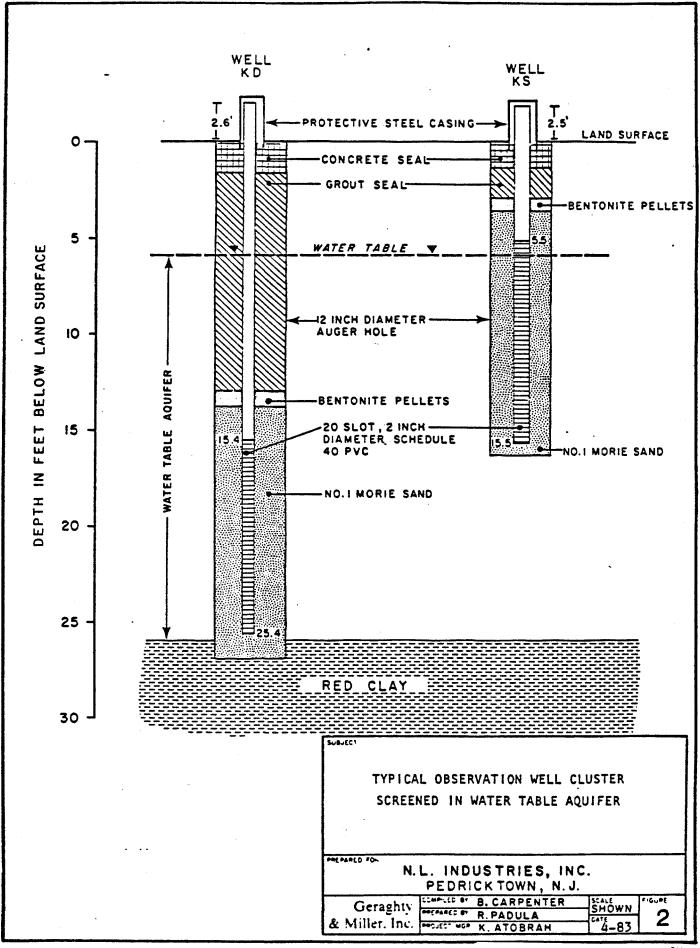


Table 1. Construction Details of Official Monitoring Wells at NL Industries, Pedricktown New Jersey, Plant Site.

Well No.	Elevation of Measuring Point (feet above mean sea level)	Total Depth Drilled (feet below land surface)	Screened Interval (feet below land surface)	Height of Measuring Point (feet above land surface)	Screen Slot Size (thousandths of an inch)
1R	13.32	35.5	4 - 32	4.0	20
2R2	9.14	25	13 - 20	2.2	20
3R	14.10	33	4 - 33	2.7	20
4R	14.80	29	9 - 21	2.7	20
5R	10.03	35.5	7 - 16	2.0	20
6	12.23	21.5	11 - 21	2.5	. 40
8R	16.55	124.5	101 -108	2.9	18
9R2	16.73	73	53 - 61	2.8	18
AR	11.39	34.5	2.5- 32.5	2.5	20
BR	8.88	45	31 - 37	2.3	18
CR2	15 96	45	25 - 31	2.8	20
10	13.72	82	42.0-72.0	2.0	20
11	9.25	59	33.2-53.2	1.8	20

Note: All wells are 4-inch diameter.

Table 2. Gonstruction Details of Observation Wells Installed in November-December 1982, at NL Industries, Pedricktown, New Jersey Plant Site.

Well No.	Elevation of Measuring Point (feet above mean sea level)	Total Depth Drilled (feet below land surface)	Screened Interval (feet below land surface)	Height of Measuring Point (feet above land surface)
HD	16.73	41	23.8 - 38.8	2.6
HS	16.83	25	9.4 - 24.4	2.6
ID	15.24	42 .	18.6 - 33.6	2.6
IS	15.41	16	5.5 - 15.5	2.5
JD	12.08	27	15.1 - 25.1	2.9
JS	11.95	15	4.4 - 14.4	2.6
KD	10.70	29	15.4 - 25.4	2.6
KS	10.51	16	5.5 - 15.5	2.5
LD	10.89	19	9.7 - 16.7	2.3
LS	10.74	11	3.9 - 10.9	
MD	8.37	19	9.6 - 17.6	2.0
MS	9.83	. 10	3.2 - 10.2	2.8
ND	10.35	22 .	11.9 - 21.9	2.1
NS	11.30	14	4.2 - 14.2	2.6
0D	11.44	37	19.5 - 34.5	3.0
0S	10.92	20	3.8 - 18.8	2.2
PD	10.25	30	16.8 - 26.8	3.2
PS	9.14	18	7.9 - 17.9	2.1
QD	10.19	25	11.5 - 21.5	2.5
QS	10.52	13	2.4 - 12.4	2.6
RD	13.62	41	25.0 - 35.0	2.0
RS	13.84	20	5.0 - 20.0	2.0
SD	11.45	30	15.0 - 27.0	2.5
SS	10.76	15	5.0 - 15.0	2.0
T2	11.34	27	7.6 - 22.6	2.4
T4*	11.09	23	8.0 - 23.0	2.0

<sup>\*) 4-</sup>inch diameter schedule 80 PVC screened with 20 slot.

_ Description	Depth (feet below land surface)	Thickness (feet)
Descripcion	12110 00110007	(1000)
Well HD		
Sand, brown, fine; with silt Sand, gray-white, fine to medium; with	0 - 4	4
silt Silt, gray-white; with lenses of	4 - 7	3
pink-red, white, mottled silty clay, gray-white silty fine sand and red-brown silty fine sand	7 - 21.5	14.5
Sand, yellow-brown, fine; with silt Sand, gray-white, fine to medium; with silt and occasional gray silty	21.5 - 29	7.5
clay lenses	29 - 38	9
Silt, gray-white, brown, mottled; with clay	38 - 41	3
Well ID		
Sand, brown, fine; with silt and Sand, light brown, fine; with silt and	0 - 3	3 .
lenses of fine silty sand Sand, gray-white, fine to medium; with	. <b>3 - 8</b>	5
silt	8 - 11.5	3.5
Silt, dark gray; with clay and lenses of fine sand	11.5 - 19	7.5
Sand, gray-white, fine to medium Sand, gray-white, fine to coarse; with	19 - 24	5
some silt and clay	24 – 28	4
Sand, gray-white, fine to medium; with some silt and clay lenses	28 - 37.5	9.5
Clu, red-pink and brown, mottled; with silt	37.5 - 41.5	4

Description	Depth (feet below land surface)	Thickness (feet)
Well JD		
Sand, light brown, fine to medium Sand, gray-white, fine to medium; with silt and occasional lenses of	0 - 4	4
clayey sand Sand, gray—white, fine to coarse; with silt, and occasional lenses of	4 - 9	5
sandy clay Sand, gray-white, fine to coarse; with silt, occasional lenses of sandy	9 - 20	11
clay and fine gravel Sand, white, brown, mottled, very fine; with clay, and lenses of sandy	20 – 23	3
clay, with silt Clay, red-pink, white, mottled; with silt	23 - 25 25 - 27	2 2
Well KD		
Silt, dark brown; with fine sand and organic matter Sand, gray, fine; with some silt Sand, gray-white, fine to coarse; with a lense of green-brown mottled	0 - 5.5 5.5 - 8.5	5.5 3
very fine clayey sand with some silt Sand, gray-white, fine to medium; with	8.5 - 18	9.5
occasional lenses of sandy silt Clay, red-pink, white, mottled; with	18 - 26.5	8.5 2.5
some silt	26.5 - 29	2.3
Well LD		
Topsoil Sand, light brown, fine Sand, brown, fine; with some silt	0 - 0.5 0.5 - 4 4 - 9	0.5 3.5 5
Sand, gray-white, fine to medium; with silt and lenses of sandy clay Clay, red-pink, brown, white, mottled;	9 - 17	8
with some silt	17 - 19	2

- Description	Depth (feet below land surface)	Thickness (feet)
Well MD		
Topsoil Sand, light brown, fine; with silt Silt, gray; with very fine sand and	0 - 0.5 0.5 - 6	0.5 5.5
some clay Sand, brown, fine; with silt Sand, yellow-brown, fine to medium;	6 - 10.5 10.5 - 14	4.5 3.5
with some silt	14 - 17	3
Silt, red-pink; with come clay and very fine sand	17 - 19	2
Well ND		
Topsoil Sand, light brown, fine to medium;	0 - 0.5	0.5
with a trace of silt	0.5 - 5	4.5
Sand, gray-brown, fine to medium; with a trace of silt	5 - 9	4
Sand, gray-white, fine; with silt and clay and lenses of sandy clay Sand, gray-white, fine to coarse; with silt and clay and occasional	9 - 14	5
lenses of sandy clay Clay, red-pink and white, mottled; with	14 - 21.5	7.5
silt	21.5 - 22	0.5
Well OD		
Topsoil Sand, light brown, fine to medium; with	0 - 0.5	0.5
some silt	0.5 - 7.5	7
Sand, brown, fine to coarse; with a gray-violet silt lense Sand, gray-white, fine to medium; with some silt and occasional lenses of	7.5 - 11.5	4
sandy clay Sand, gray-white, fine to coarse; with some fine gravel and lenses of	11.5 - 24	12.5
sandy clay Clay, red-pink; with some silt	24 - 35.5 35.5 - 37	11.5 1.5

	Depth			
Donominhion	(feet below	Thickness		
Description	land surface)	(feet)		
Well PD				
Topsoil	0 - 0.5	0.5		
Sand, brown, fine to medium; with silt Silt, gray; with very fine sand and	0 - 0.5 0.5 - 3	2.5		
some clay Sand, brown, fine to medium; with silt	3 - 4.5	1.5		
and a lense of coarse sand with occasional fine gravel Sand, red-brown, medium to coarse; with	4.5 - 9	4.5		
lenses of sandy clay  Sand, gray, fine to medium; with a	9 - 15.5	6.5		
trace of fine gravel Sand, white-gray, fine to medium; with silt and occasional lenses of	15.5 - 19	3.5		
clayey silt with fine sand Clay, red-pink, white, mottled	19 - 28 28 - 30	9 2		
Well QD				
Sand, brown, fine to medium; with silt Sand, gray-white, fine to medium; with	0 - 8	8		
silt Sand, gray-white, fine to coarse; with	8 - 14	6		
silt Clay, red-pink, white, mottled: with	14 - 22	8		
silt	22 - 25	3		
Well RD				
Topsoil Sand, light brown, fine to medium; with	0 - 0.5	0.5		
a trace of silt Sand, gray, fine to medium; with a	0.5 - 9	8.5		
trace of silt Clay, gray; with some silt and occa- sional lenses of fine silty	9 - 16.5	7.5		
sand Silt, red, with some white mottling;	16.5 - 34	17.5		
sand, very fine and some clay	34 - 40.5	6.5		

Description	Depti (feet be land sur	Thickness (feet)	
Well SD			
Sand, brown very fine; with some silt, trace of clay	0 -	3	3
Sand, brown, fine to medium; with some silt, plastic debris from battery casing	3 -	14	11
Sand, gray-white, fine to coarse; with silt Sand, gray-white, fine to coarse; with	14 -	20	6
occasional lenses of silty clay Clay, red-pink, white, mottled; with	20 - 3		<b>.</b>
some silt	28 - 1	30	2
Well T2			•
Fill, sand, brown, fine to medium; silt, black, with plastic pieces			
from batteries Sand, brown, fine to medium	0 -	3	3 6 ·
Sand, gray, fine to medium; with some	, -	7	. •
silt and a trace of fine gravel Sand, gray, fine; with some silt and a	9 -	14	5
trace of fine gravel Sand, gray-white, fine to coarse; with	14 – 1	<b>2</b> 2	8
some fine gravel Clay, red-pink, white, mottled; with	22 -	23	1
silt	23 -	27	4

EXHIBIT D

NSNJ Pedricktown Landfill Leachate

		hase "A"		hase "B"	Fhase "A"			hase "B"
•	- Pri	mary Sump	Pri	mary Sump	Seco	ndary Sump	Seca	ndary Sump
Parameter	'n	Max	n	Max	n	Max	٠m	Max
Antimony St	7	6.6	7	0.48	i	0.49		
Arsenic As	3	38. 3	1	227	. 1	0.006	7	0.49
Arsenic Filt.	2	2.7	1	72.1	1.	0.007	1	51.7
Barium Ba	1	1					1	60. E
Cadmium Cd	4	0.31	1	0.06	1	0.01	. 1	0.06
Cadmium Filt.	2	0.3	1	0.05	1	0.01	1	0.05
Chloride Cl	7	19300	- 7	630	1	154	6	525
Iron Fe	8	12300	7	184	1	1.1	7	9.5
Iron Filt.	1	8100						
Lead Pb	9	12.3	8	24.5	2	0.49	8	2.95
Lead Filt.	1	0.04	1	0.38	1	0.5	. 1	0.33
Manganese Mn	4	15.8	1	7.5	1	. 23	1	0.67
Manganese Filt.	. 5	13	1	0.07	1	22	1	0.67
Selenium Se	4	0.27	1	0.58	1	0.009	1	0.75
Selenium Filt.	2	0.041	1	0.44	1	0.011	1	0.61
Sulfate SD4	4	39100	- 1	20000	1	2730	1	33300
Tin Sn	6	13	7	1.4	1	0.5	7	2
T. O. C.	1	150	. 1	2170	1	27	1	2520
B. O. D.	. ε	9200	7	210	1	3	7	٤
C. O. D.	6	17100	7	560	1	49	7	· 97
Hardness	. 6	564	7	1625	1	790	6	1100
pН	10	12.7(1.7)	В	11 (4.6)	2	6.3(5.2)	8	9.4(5.3)
Phenols	€.	16.1	7	C. 78	:	0.002	7	0.128
T.D.S.	10	159000	8	66400	2	4300	8	64700
Turbidity	10	920	8	250	2	31	8	170

### Notes

<sup>-</sup> If two pH values are recorded, the first value is the maximum pH and the second, in parentheses, is the minimum value.

<sup>-</sup> Samples collected by NL Industries, Inc.; analysis conducted by Century Environmental Labs, Inc., Thorofare, NJ.

EXHIBIT E

Table 3. Summary of Water-Level Elevation Data, NL Industries, Pedricktown, New Jersey.

	Elevation of	Wate	er-Level Ele	evation (fee	et above or	below mea	an sea leve	el)
Well No.	Measuring Point	12-9-82	12-15-82	12-21-82	12-28-82	1-6-83	1-11-83	3-8-83
1R	13.32	4.60	4.56	4.87	4.84	4.89	5.22	6.27
2R2	9.14	2.31	2.32	2.48	2.50	2.58	2.71	3.22
3R	14.10	2.83	2.85	3.02	3.03	3.09	3.20	_
4R	14.80	3.08	3.06	3.24	3.27	3.23	3.28	4.21
5R	10.03	3.39	3.33	3.65	3.57	3.87	4.36	5.77
6	12.23	3.77	3.74	3.96	3.91	4.03	4.36	4.78
8R	16.55	-8.63	-6.38	-8.35	-8.49	-6.95	-7.82	-2.99
9R2	16.73	-3.52	-2.77	-3.42	-3.25	-2.60	-3.01	6.50
10	13.72	-2.68	-2.30	-2.48	-2.23	-2.18	-2.27	-2.00
11	9.25	4.77	3.80	4.00	4.04	4.17	4.37	5.01
AR	11.39	4.14	4.13	4.36	4.39	4.44	4.62	6.45
BR	8.88	3.46	3.46	3.66	3.68	3.83	3.99	4.68
CR2	15.96	2.75	2.88	3.07	3.27	3.39	3.39	-
HĐ	16.73	2.75	2.86	3.04	3.25	3.37	_ 3.38	_
HS	16.83	2.78	2.87	3.05	3.26	3.36	3.37	_
ID	15.24	4.05	4.04	4.24	4.35	4.34	4.45	6.39
IS -	15.41	6.42	6.46	6.68	6.82	6.82	6.99	9.74
JD	12.08	4.29	4.20	4.55	4.48	4.54	4.86	6.58
JS	11.95	4.29	4.24	4.57	4.54	4.57	4.88	-
KD	10.70	3.98	3.98	4.22	4.22	4.31	4.57	6.08
KS	10.51	4.06	4.09	4.31	4.32	4.38	4.66	6.18
LD	10.89	3.76 °	3.74	4.06	4.00	4.00	4.23	5.53
LS	10.74	3.81	3.77	4.14	4.09	4.08	4.40	5.84
MD	8.37	2.45	2.51	2.69	2.70	2.77	2.88	3.51
MS	9.83	2.54	2.56	2.71	2.73	2.82	3.07	3.55
ND	10.35	3.15	3.09	3.32	3.27	3.41	3.62	4.23
NS	11.30	3.17	3.14	3.37	3.33	3.44	3.63	4.30
0D	11.44	3.46	3.44	3.65	3.58	3.74	4.09	4.51
OS	10.92	3.58	3.54	3.78	3.71	3.84	4.22	4.70
PD	10.25	3.63	3.65	3.80	3.80	3.92	4.11	4.72
PS	9.14	3.63	3.65	3.80	3.80	3.93	4.12	4.72
QD	10.19	3.89	-	4.17	4.14	4.24	4.57	5.48
QS	10.52	3.96	3.94	4.19	4.19	4.27	4.61	5.54
RD	13.62	4.33	4.90	5.09	5.23	4.74	4.66	6.88
RS	13.84	5.68	5.64	5.91	5.99	5.99	6.22	7.46
SD	11.45	3.66	3.72	3.92	3.97	3.83	4.07	5.29
SS	10.76	3.90	3.89	4.08	4.14	4.17	4.38	-
T2	11.34	3.79	3.77	4.00	3.97	4.05	4.39	5.42
T4	11.09	3.77	3.79	3.99	3.94	4.01	4.37	5.06

Note: All wells measured from top of PVC

Excerpt from: Geraghty & Miller, Inc. May 1983. "Hydrogeologic Study and Design of Groundwater Abatement System at NL Industries, Inc. Pedricktown, New Jersey Plant Site."